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THERMO-STRUCTURAL ANALYSIS MANUAL

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FOREWORD

This Manual was prepared by the Structural Research and Development Group, Structures Section, Research and Development Division of Republic Aviation Corporation. The work was initiated under Contract AF33(616)-6066 in the 750A Applied Research Program, the Mechanics of Flight, Project No. 1367, Structural Design Criteria, and Task No. 14002, "Structural Analysis Methods". The work is now documented under Task 136710. This work was initiated under the direction of the Structural Analysis Unit, Structures Branch, Aircraft Laboratory, Directorate of Laboratories, Wright Air Development Center.* Mr. I. Winnegrad acted initially as project engineer and was succeeded by Mr. C. Richard. The Manual was completed under the direction of the Structural Analysis Unit, Configuration Research Section, Structures Branch, Flight Dynamics Laboratory, Deputy Commander/Technology, Aeronautical Systems Division, with Mr. G. E. Maddux as Project Engineer.

The work was coordinated and supervised by Dr. R. S. Levy, Head of the Structural Research and Development Group. His valuable suggestions and criticisms are gratefully acknowledged as are those of the following personnel of the Applied Research and Development Division of Republic Aviation Corporation: Mr. A. Alberi, Acting Manager of Technical Engineering; Mr. C. Rosenkranz, Acting Chief Structures Engineer; and Mr. C. Meissner, Principal Structures Engineer.

NOTES ON USING THE MANUAL

This Manual consists of five basic sections, divided into numbered sub-sections and paragraphs. For simplicity in cross-referencing material in the text, all portions of the Manual designated with a two-tier number (e.g., 1.1) are considered sub-sections, and all portions designated by numbers of three or more tiers (e.g., 1.1.1 or 1.1.1) are considered paragraphs.

Throughout the Manual, the numbered paragraphs (or sub-sections) have been used as the basis for numbering figures, tables, and equations, with new sequences beginning with each numbered paragraph. Figure and table numbers consist of an appropriate paragraph number, followed by a sequence number for the particular figure or table. For convenience the paragraph designations have been omitted from the equation numbers. When an equation from another paragraph is cited in the text, the number of the paragraph in which that equation occurs is also cited. When a paragraph number is not given in conjunction with the citation of an equation, it is to be assumed that the equation is included in the paragraph in which the citation occurs.

References are listed at the end of those sections which have more than one reference. In addition, each section contains its own complete table of contents and list of symbols.

*Now under direction of Flight Dynamics Laboratory, Directorate of Aeromechanics, Aeronautical Systems Division.

ABSTRACT

This second volume of the Thermo-Structural Analysis Manual considers additional problems in the field of thermal and mechanical stress analysis not fully treated in Volume I. Special emphasis is given to nonlinear analysis of beams and plates and to axisymmetric thermo-elastic analysis of thin shells. Following the format of Volume I, nondimensional graphs, formulas and tables are developed where feasible. For clarification of the analytical techniques and the use of the numerical data, illustrative examples are given.

The following problems are treated in five individual sections:

- (1) Large deflection analysis of straight elastic beams with axial end restraint and axial end loads coupled with transverse leading and temperature,
- (2) Approximate determination of the axial end loads and deformations in heated beam columns with initial eccentricities,
- (3) Approximate solutions for the buckling of eccentric columns accommodating nonlinear stress-strain laws,
- (4) Axisymmetric large deflections of circular plates subjected to thermal and mechanical loads.
- (5) Axisymmetric thermo-elastic analysis of thin shells.

PUBLICATION REVIEW

This publication has been reviewed and is approved.

FOR THE COMMANDER:

P. F. HOKNER

Chief, Structures Branch Flight Dynamics Laboratory

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INTRODUCTION

This Volume II of the Manual is an extension of the basic work presented in Volume I and covers the following special problem areas in heated beams, plates and shells:

- (1) Nonlinear beam analyses including the effects of initial eccentricities and nonlinear material properties.
- (2) Elastic circular plates loaded and heated axisymmetrically, considering the effects of large deflections
- (3) The axisymmetric thermoelastic analysis of shells developed in Volume I is generalized, removing the restrictions on geometry and including the effects of arbitrary temperature variations through the thickness.

The material is presented in the form of five independent reports or sections.

The emphasis has been placed on the development of analytical techniques and formulas with their corresponding physical interpretations. Where feasible, the investigations are self-contained, starting with fundamental theoretical considerations and notions in the field of static thermo-structural analysis. However, the reader may find it useful to refer to Volume I where a systematic development of the basic concepts is given.

Brief summaries of the five sections of this volume of the Manual follow.

Section 1. Beam Columns Subjected to Elevated Temperature and Mechanical Loads

This section treats beam columns with axial end restrain so a lond loads coupled with transverse loads and temperature gradients. Numerical results, a weet within the framework of large deflection beam theory are tabulated for certain cases.

Section 2. Approximate Solution for an Axially Restrained Column Subjected to Elevated Temperature and Lateral Load

This section presents an approximate method of solution of the heated beam column problem. The method is amenable to problems involving spanwise variations of load thermal gradients and stiffness and permits treatment of initial eccentricities.

Section 3. Approximate Solution for the Buckling of Eccentric Columns

This section develops and presents nondimensional curves to predict column buckling leads. Nonlinear stress-strain relationships are accommodated as well as lateral loads and initial eccentricities or thermally induced deformations.

Section 4. Axisymmetric Large Deflections of Circular Plates Subjected to Thermal and Mechanical Loads

The meaning of membrane stresses and bending is considered for axially restrained circular plates subjected to heat and load. The differential equations governing the axisymmetric case are derived and an iterative digital scheme is used to obtain numerical results for a special case over a wide range of temperature and load parameters.

Section 5. Axisymmetric Stresses and Deflections in Shells Due to Thermal and Mechanical Loads

The general equation for linear elastic analysis of axisymmetric shell problems is developed including the effects of temperature and load. The special cases of conical and cylindrical shells are developed and a numerical example is given for a cylinder with temperature gradients through the thickness and along the length.

SECTION 1

BEAM COLUMNS SUBJECTED TO ELEVATED TEMPERATURE AND MECHANICAL LOADS

by

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SECTION 1

BEAM COLUMNS SUBJECTED TO ELEVATED TEMPERATURE AND MECHANICAL LOADS

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SECTION 1

BEAM COLUMNS SUBJECTED TO ELEVATED TEMPERATURE

AND MECHANICAL LOADS

1.1. SUMMARY

This report considers the nonlinear analysis (large deflections) of beams with axial end restraints and axial end loads coupled with transverse loading and temperature.

In order to keep the number of parameters within practical limits the following conditions are investigated:

- (1) Distributed transverse loads are uniform over the span while concentrated loads are at the midspan.
- (2) The temperature varies linearly through the depth and is constant in a spanwise direction.
- (3) The beam is assumed to be simply supported at its ends for bending and elastically restrained axially (Figure 1.3-1).

Tables of numerical results in nondimensional form are presented for the cases of zero and full axial end restraint in rectangular beams. These tables may be used to determine maximum deflections and bending moments.

1. 2 INTRODUCTION

In structural analysis beam columns differ from simple beams in that the addition of axial end loads and restraints interact with transverse loads in a nonlinear manner, thus invalidating the principle of superposition (Reference 1-1).

When the axial end loads are specified and the beam is unrestrained axially, the solution for bending moments and deflections are obtained, in general, by solving a linear, non-homogeneous differential equation with constant coefficients subject to appropriate boundary conditions. If, in addition, the beam is restrained axially, the total end load is an unknown and an additional compatibility relation must be employed.

The analysis presented considers a beam of constant cross section for which the Bernoulli-Euler assumption of classical beam theory is employed. This implies that plane sections perpendicular to the centroidal axis before bending remain plane and perpendicular to the deflected centroidal axis. It is further assumed that the material behavior is linearly elastic.

It can be shown that the seemingly approximate technique used below (Sub-section 1.3) yields results which are identical to those obtained by a moderately large deflectic, analysis in which nonlinear strain-displacement relations are employed.

1.2 (Cont'd)

The following symbols are used throughout this section:

b	Width of rectangular beam
h	Depth of beam
x	Spanwise coordinate
$\frac{\mathbf{y}}{\mathbf{y}}$	Vertical deflection
$\overline{\mathbf{y}}$	Nondimensional central deflection
A	Cross sectional area
E	Young's modulus
H	Magnitude of axial load in beam
I	Moment of inertia
2K	Spring stiffnes, of axial end restraint
L	Half beam length
M	Bending moment
$\overline{\mathbf{M}}$	Nondimensional central bending moment
P	Known applied axial end load
$\overset{2}{\sim}$ Q	Concentrated midspan transverse load
ଭୂ, ଭୂ	Nondimensional concentrated midspan load
T_i, T_o	Temperatures at lower and upper beam faces, respectively
$\begin{array}{c} 2Q \\ \widetilde{Q}, \overline{Q} \\ T_i, \overline{T}_o \\ T_d, \overline{T}_d \end{array}$	Nondimensional temperature differences between upper and lower beam faces
$\overline{ extbf{T}}$	Nondimensional average temperature
W	Intensity of uniformly distributed transverse load
$\widetilde{\widetilde{W}}, \overline{\widetilde{W}}$	Nondimensional uniformly distributed load
α	Coefficient of linear thermal expansion
β	Ratio of distance from lower beam face to centroidal axis to the total depth
•	Ţ <u>H</u>
λ	$^{\pm}\sqrt{EI}$
$\overline{\overline{\lambda}}$	\L, nondimensional
σ	otress

SUBSCRIPTS

M	Due	to	mechanical loads
T	Due	to	temperature

1.3 DERIVATION OF BASIC EQUATIONS

The beam (Figure 1.3-1) is referred to a rectangular coordinate system in which transverse deflections y and distances along the centroidal axis are measured from a point on the undeflected centroidal axis at mid-span. Specified axial end loads are denoted by P and the elastic axial restraints are shown schematically as springs with stiffness 2K. We assume uniform distributed loads of intensity W and a mid-span concentrated load of magnitude 2Q. The temperature variation through the thickness is linear, varying from $T_{\hat{i}}$ at the lower extreme fiber to $T_{\hat{o}}$ at the upper extreme fiber.

Since the temperature distribution varies linearly with respect to Cartesian coordinates it produces no stresses in an externally unrestrained beam (Reference 1-2). In this case, the stress-free thermal curvature is given by

$$y_{T}^{ij} = \frac{\alpha(T_{O} - T_{j})}{h} . \tag{1}$$

1.3 (Cont⁸d)

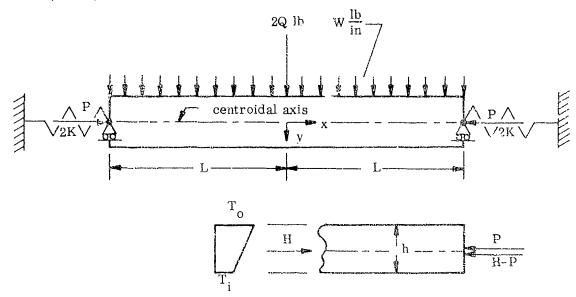


FIGURE 1, 3-1 BEAM COLUMN - MODEL USED FOR ANALYSIS

We now consider that the external specified and redundant mechanical loads are applied to this thermally bent beam. Designating the additional curvatures produced by these mechanical loads as y_M^n , it follows that

$$y_{M}^{"} = \frac{-M}{EI} = \frac{1}{EI} \left[\pm Hy - (WL + Q) (L - x) + \frac{W}{2} (L - x)^{2} \right],$$
 (2)

where positive moments cause compression in the outer fibers, y is the total deflection, and H is the magnitude of the axial load in beam*. The negative and positive signs in the first term of the right hand side of Eq. (2) refer to compression and tension respectively. Adding Eqs. (1) and (2) and rearranging yields

$$y'' + \frac{Hy}{EI} = \frac{W}{2EI} (L - x)^2 - \frac{(WL + Q)(L - x)}{EI} + \frac{\alpha}{h} (T_0 - T_i)$$
 (3)

Due to symmetry it is only necessary to consider one half of the beam, so that the differential equation given by Eq. (3) applies in the interval $0 \le x \le L$ and y(x) is an even function, i.e., y(x) = y(-x). The boundary conditions are given by

$$y'(0) = y(L) = 0$$
 (4)

1,4 SOLUTION OF EQUATIONS

The complete plutions of Eqs. (3) and (4) of Sub-section 1.3 for the cases of axial compression and tension, respectively, are written as follows.

^{*} H is always taken as positive, regardless of whether the axial load is tensile or compressive.

1, 4 (Cont*d)

(1) Axial Compression

$$y = \frac{Q}{EI\lambda^3} \frac{\sin\lambda (L-x)}{\cos\lambda L} + \frac{\cos\lambda x}{\lambda^2\cos\lambda L} \left[\frac{W}{EI\lambda^2} - \frac{\alpha}{h} (T_o - T_i) \right]$$

$$+ \frac{Wx^2}{2EI\lambda^2} + \frac{Qx}{EI\lambda^2} + \frac{1}{\lambda^2} \left[\frac{\alpha}{h} (T_o - T_i) - \frac{QL}{EI} - \frac{WL^2}{2EI} - \frac{W}{EI\lambda^2} \right] .$$
(1a)

(2) Axial Tension

$$y = \frac{Q}{EI\lambda^{3}} \frac{\sinh\lambda (x - L)}{\cosh\lambda L} + \frac{\cosh\lambda x}{\lambda^{2}\cosh\lambda L} \left[\frac{W}{EI\lambda^{2}} + \frac{\alpha}{h} (T_{o} - T_{i}) \right]$$

$$-\frac{Wx^{2}}{2EI\lambda^{2}} - \frac{Qx}{EI\lambda^{2}} - \frac{1}{\lambda^{2}} \left[\frac{\alpha}{h} (T_{o} - T_{i}) - \frac{QL}{EI} - \frac{WL^{2}}{2EI} + \frac{W}{EI\lambda^{2}} \right] .$$
(1b)

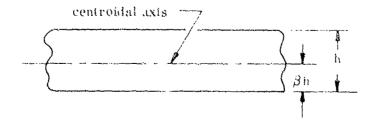
where

$$\lambda = -\sqrt{\frac{H}{EI}}$$
 for compression,
$$\lambda = +\sqrt{\frac{H}{EI}}$$
 for tension.

The solution is not yet complete since, in general the axial end load H and hence λ is unknown. An additional compatibility relationship must be employed to evaluate this quantity. Such a relationship is obtained by noting that the change τ the spanwise distance between the beam ends due to bending, thermal expansion and mechanical axial strains must be equal to the total change in length of the axial end restraints. This condition can be expressed by

$$\frac{1}{2} \int_{-0}^{L} (y^{i})^{2} dx + \frac{HL}{AE} - \omega L \left\{ \beta T_{o} + (1 - R) T_{i} \right\} + \frac{(EH - P)}{2K} = 0 , \qquad (3)$$

where positive and negative signs associated with H refer to compression and tension respectively and β defines the location of the centroidal axis (Figure 1.4-1).



1.4 (Cont*d)

Substitution of Eqs. (1a) and (1b) into (3) and simplifying yields for the compression case

$$\frac{\widetilde{Q}^{2}}{\overline{\lambda}^{5}} \left[(\sin \overline{\lambda}) \left(\cos^{2} \frac{\overline{\lambda}}{2} - \frac{5}{2} \right) + \frac{3\overline{\lambda}}{2} \right]
+ \left[\frac{1}{2} - \frac{\sin 2\overline{\lambda}}{4\overline{\lambda}} \right] \left[\frac{1}{\cos^{2} \overline{\lambda}} \right] \left[\frac{\widetilde{W}}{\overline{\lambda}^{3}} + \frac{\widetilde{Q} \sin \overline{\lambda}}{\overline{\lambda}^{2}} - \frac{\widetilde{T}_{d}}{\overline{\lambda}} \right]^{2}
+ \frac{\widetilde{W}^{2}}{3\overline{\lambda}^{4}} - \left[\frac{\widetilde{W}}{\overline{\lambda}^{3}} + \frac{\widetilde{Q} \sin \overline{\lambda}}{\overline{\lambda}^{2}} - \frac{\widetilde{T}_{d}}{\overline{\lambda}} \right] \left[\frac{3}{\overline{\lambda}^{3}} \cos \overline{\lambda} \sin^{4} \left(\frac{\overline{\lambda}}{\lambda} \right) \right]
+ \frac{\widetilde{Q}\widetilde{W}}{\overline{\lambda}^{4}} \left[1 - 2 \left(\frac{\sin \overline{\lambda}}{\overline{\lambda}} + \frac{\cos \overline{\lambda}}{\overline{\lambda}^{2}} - \frac{1}{\overline{\lambda}^{2}} \right) \right]
+ \frac{2\widetilde{W}}{\overline{\lambda}^{4} \cos \overline{\lambda}} (\sin \overline{\lambda} - \overline{\lambda} \cos \overline{\lambda}) \left[\frac{\widetilde{W}}{\overline{\lambda}^{3}} + \frac{\widetilde{Q} \sin \overline{\lambda}}{\overline{\lambda}^{2}} - \frac{\widetilde{T}_{d}}{\overline{\lambda}} \right]
+ \left[\frac{2I}{AL^{2}} + \frac{EI}{KL^{3}} \right] \quad \overline{\lambda}^{2} = \frac{P}{KL} + 2\alpha \left[\beta T_{0} + (1 - \beta) T_{1} \right] ,$$
(4a)

and for the case of axial tension:

$$\frac{\tilde{Q}^{2}}{\bar{\chi}^{5}} \left[\left(\sinh \bar{\lambda} \right) \left(\cosh^{2} \frac{\bar{\lambda}}{2} - \frac{5}{2} \right) + \frac{3\bar{\lambda}}{2} \right]$$

$$- \left[\frac{1}{2} - \frac{\sinh 2\bar{\lambda}}{4\bar{\lambda}} \right] \left[\frac{1}{\cosh^{2}\bar{\lambda}} \right] \left[\frac{\tilde{W}}{\sqrt{3}} - \frac{\tilde{Q} \sinh \bar{\lambda}}{\bar{\lambda}^{3}} + \frac{\tilde{T}_{d}}{\bar{\lambda}} \right]^{2}$$

$$+ \frac{\tilde{W}^{2}}{3\bar{\lambda}^{4}} + \left[\frac{4\tilde{Q} \sinh^{4}\bar{\lambda}}{\bar{\chi}^{3} \cosh \bar{\lambda}} \right] \left[\frac{\tilde{W}}{\bar{\chi}^{3}} - \frac{\tilde{Q} \sinh \bar{\lambda}}{\bar{\lambda}^{2}} + \frac{\tilde{T}_{d}}{\bar{\chi}^{2}} \right]$$

$$+ \frac{\tilde{Q} \tilde{W}}{\bar{\lambda}^{4}} \left[1 - 2 \left(\frac{\sinh \bar{\lambda}}{\bar{\lambda}} - \frac{\cosh \bar{\chi}}{\bar{\chi}^{2}} + \frac{1}{\bar{\chi}^{2}} \right) \right]$$

$$+ \frac{2\tilde{W}}{\bar{\chi}^{4} \cosh \bar{\lambda}} \left(\sinh \bar{\lambda} - \bar{\lambda} \cosh \bar{\lambda} \right) \left[-\frac{\tilde{W}}{\bar{\chi}^{3}} - \frac{\tilde{Q} \sinh \bar{\lambda}}{\bar{\lambda}^{2}} + \frac{\tilde{T}_{d}}{\bar{\lambda}} \right]$$

$$- \left(\frac{21}{\Lambda L^{2}} + \frac{E1}{KL^{3}} \right) \quad \bar{\lambda}^{2} = \frac{P}{KL} + 2\sigma \left[\beta T_{0} + (1 - \beta) - T_{1} \right] ,$$

$$(4b)$$

1.4 (Cont'd)

where

$$\frac{QL^{2}}{EI} = \widetilde{Q}$$

$$\frac{WL^{3}}{EI} = \widetilde{W}$$

$$\lambda L = \overline{\lambda} = + \sqrt{\frac{HL^{2}}{EI}} \text{ for tension}$$

$$= -\sqrt{\frac{HL^{2}}{EI}} \text{ for compression}$$

$$\frac{\alpha L (T_{0} - T_{i})}{h} = \widetilde{T}_{d} .$$

Equations (4a) and (4b) are transcendental equations from which $\tilde{\lambda}$ may be determined. Equations (1a) and (1b) then yield the deflections for the known values of $\tilde{\lambda}$. In general, Eqs. (4) are difficult to solve except by graphical or trial and error procedures. In addition, presentation of numerical results for the large number of parameters appearing in this general problem is cumbersome. For these reasons, the number of parameters to be used in the presentation of numerical results have been reduced by restricting considerations to the following:

- (1) The beam cross section is rectangular and therefore $\beta = \frac{1}{2}$.
- (2) The beam is prevented from moving axially at its ends $(K \rightarrow *)$ or free to move axially (K = 0).
- (3) The transverse load is either uniform over the span or concentrated at the mid-spat.

1.5 NUMERICAL RESULTS FOR BEAMS OF RECTANGULAR CROSS SECTION

CASE A - Uniform transverse load over the span with ends rigidly restrained axially:

The beam is shown schematically in Figure 1.5-1. For this case, we substitute $\widetilde{Q} = 0$, $\beta = \frac{1}{2}$, and $\frac{1}{K} = 0$ into Eqs. (1) and (4) of Sub-section 1.4. D—ping nondimensional quantities:

$$\widetilde{W} = \frac{12 W}{Eb} \left(\frac{L}{h}\right)^{4}$$

$$\widetilde{\chi} = \pm \sqrt{\frac{HL^{2}}{EI}} \quad \text{(tension positive)}$$

1.5 (Cont'd)

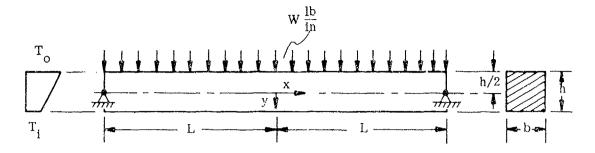


FIGURE 1.5-1 RECTANGULAR BEAM WITH UNIFORM LOAD OVER THE SPAN AND ENDS RIGIDLY RESTRAINED AXIALLY

$$\overline{T}_{d} = \alpha \left(\frac{L}{h}\right)^{2} \quad \left(T_{o} - T_{i}\right)$$

$$\overline{T} = \alpha \left(\frac{L}{h}\right)^{2} \quad \left(T_{o} + T_{i}\right)$$

$$\overline{y} = \left[\frac{y}{h}\right]_{X = 0}$$

$$\overline{M} = \left[\frac{12ML^{2}}{Ebh^{4}}\right]_{X = 0}$$
(1)

These equations yield

$$\left[\frac{1}{2} - \frac{\sin 2\bar{\lambda}}{4\bar{\lambda}} \right] \left[\frac{1}{\bar{\lambda}^2 \cos^2 \bar{\lambda}} \right] \left[\frac{\bar{W}}{\bar{\lambda}^2} - \bar{T}_d \right]^2 + \frac{\bar{W}^2}{3\bar{\lambda}^4}
- \frac{2\bar{W}}{\bar{\lambda}^5 \cos \bar{\lambda}} \left(\sin \bar{\lambda} - \bar{\lambda} \cos \bar{\lambda} \right) \left[\frac{\bar{W}}{\bar{\lambda}^2} - \bar{T}_d \right] + \frac{\bar{\lambda}^2}{6} = \bar{T}
\bar{y} = \frac{1}{\bar{\lambda}^2} \left[\frac{\bar{W}}{\bar{\lambda}^2} - \bar{T}_d \right] \left[\frac{1}{\cos \bar{\lambda}} - 1 \right] - \frac{\bar{W}}{2\bar{\lambda}^2}
\bar{M} = \left[\frac{\bar{W}}{\bar{\lambda}^2} - \bar{T}_d \right] \left[\frac{1}{\cos \bar{\lambda}} - 1 \right] ,$$
(3a)

1.5 (Cont'd)

for the case of compressive end loads, while for the tensile case

$$-\left[\frac{1}{2} - \frac{\sinh 2\bar{\lambda}}{4\bar{\lambda}}\right] \left[\frac{1}{\bar{\lambda}^2 \cosh^2\bar{\lambda}}\right] \left[\frac{\bar{W}}{\bar{\lambda}^2} + \bar{T}_d\right]^2 + \frac{\bar{W}^2}{3\bar{\lambda}^4}$$

$$+ \frac{2\bar{W}}{\bar{\lambda}^5 \cosh \bar{\lambda}} \left(\sinh \bar{\lambda} - \bar{\lambda} \cosh \bar{\lambda}\right) \left[\frac{\bar{W}}{\bar{\lambda}^2} + \bar{T}_d\right] - \frac{\bar{\lambda}^2}{6} = \bar{T}$$
(2b)

$$\overline{y} = \frac{1}{\overline{\lambda}^2} \left[\frac{\overline{W}}{\overline{\lambda}^2} + \overline{T}_d \right] \quad \left[\frac{1}{\cosh \overline{\lambda}} - 1 \right] \quad + \frac{\overline{W}}{2\overline{\lambda}^2}$$

$$\overline{M} = -\left[\frac{\overline{W}}{\overline{\lambda}^2} + \overline{T}_d \right] \quad \left[\frac{1}{\cosh \overline{\lambda}} - 1 \right] \quad . \tag{3b}$$

Va' of $\bar{\lambda}$, \bar{y} , and \bar{M} for various combinations of \bar{T}_d , \bar{T} and \bar{W} are given in Table 1.5-1. This table permits the determination, either directly or by interpolation, of the axial end loads, maximum (central) deflection and maximum bending moment corresponding to specified temperatures and transverse loading. A typical case, extracted from the table is plotted in Figure 1.5-2, showing the variations in end load, central bending moment and deflection with average temperature when the temperature difference and transverse load are held constant.

The figure shows that extremely large values of \overline{T} are required to raise the compressive end load to values in the neighborhood of the critical Euler value, $\overline{\lambda}_{CR} = -\frac{\pi}{2} = -1.57$. This is due to the fact that additional beam expansions caused by increasing the average temperature are accommodated by further bending with very little change of compressive end load.

Thus, although the theoretical end load in a perfectly straight axially restrained column reaches the Euler buckling load when $\overline{T} = \frac{\pi^2}{24}$.41, values of T several orders of magnitude higher than this can actually be achieved without excessive stresses or deflections occuring. In this respect thermal buckling differs significantly from buckling caused by deadweight loads, since stresses and deflections tend to become excessively large when mechanical loads exceed the Euler buckling value.

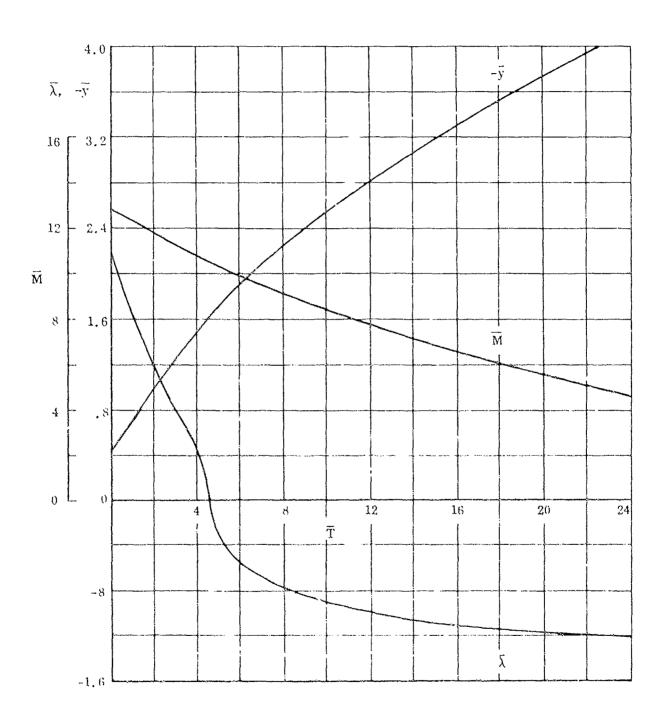


FIGURE 1.5-2 NONDIMENSIONAL END LOAD, CENTRAL DEFLECTION AND MOMENT VS. NONDIMENSIONAL AVERAGE TEMPERATURE: \overline{T}_d :12

1,5 (Cont'd)

CASE B - Concentrated transverse load at midspan with ends rigidly restrained axially For this case, we substitute $\overline{W} = 0$, $\beta = \frac{1}{2}$ and $\frac{1}{K} = 0$ into Eqs. (1) and (4) of Sub-section 1.4. Nondimensional quantities are as defined by Eq. (1) and in addition we define

$$\overline{Q} = \frac{12QL^3}{Ehh^4} \quad . \tag{4}$$

$$\frac{\overline{Q}^{2}}{\overline{\lambda}^{5}} \left[(\sin \overline{\lambda}) \left(\cos^{2} \frac{\overline{\lambda}}{2} - \frac{5}{2} \right) + \frac{3\overline{\lambda}}{2} \right]
+ \left[\frac{1}{2} - \frac{\sin 2\overline{\lambda}}{4\overline{\lambda}} \right] \left[\frac{1}{\overline{\lambda}^{2} \cos^{2} \overline{\lambda}} \right] \left[\frac{Q \sin \overline{\lambda}}{\overline{\lambda}} - \overline{T}_{d} \right]^{2}
- \frac{4\overline{Q} \sin^{4} \frac{\overline{\lambda}}{2}}{\overline{\lambda}^{4} \cos \overline{\lambda}} \left[\frac{Q \sin \overline{\lambda}}{\overline{\lambda}} - \overline{T}_{d} \right] + \frac{\overline{\lambda}^{2}}{6} = \overline{T}
\overline{y} = \frac{\overline{Q}}{\overline{\lambda}^{2}} \left(\frac{\tan \overline{\lambda}}{\overline{\lambda}} - 1 \right) - \frac{\overline{T}_{d}}{\overline{\lambda}^{2}} \left(\frac{1}{\cos \overline{\lambda}} - 1 \right)
\overline{M} = \overline{Q} \frac{\tan \overline{\lambda}}{\overline{\lambda}} - \overline{T}_{d} \left[\frac{1}{\cos \overline{\lambda}} - 1 \right],$$
(5a)

for the case of compressive end loads, while for the tensile case

$$\frac{\tilde{Q}^{2}}{\tilde{\chi}^{5}} = \left[(\sinh \tilde{\lambda}) + (\cosh^{2} \frac{1}{2} + \frac{5}{2}) + \frac{3\tilde{\lambda}}{2} \right] \\
= \left[\frac{1}{2} - \frac{\sinh 2\tilde{\lambda}}{4\tilde{\lambda}} \right] \left[\frac{1}{\tilde{\chi}^{2} \cosh^{2}\tilde{\lambda}} \right] = \left[\tilde{Q} \sinh \tilde{\lambda} - \tilde{T}_{d} \right]^{2} \\
= \frac{4\tilde{Q} \sinh^{4} \frac{\tilde{\lambda}}{2}}{\tilde{\lambda}^{4} \cosh \tilde{\lambda}} + \left[\tilde{Q} \sinh \tilde{\lambda} - \tilde{T}_{d} \right] - \tilde{T}_{d} \right] = \tilde{T} \\
\tilde{V} = -\frac{\tilde{Q}}{\tilde{\lambda}^{2}} + \left(\frac{\tanh \tilde{\lambda}}{\tilde{\lambda}} - 1 \right) + \frac{\tilde{T}_{d}}{\tilde{\chi}^{2}} \left(\frac{1}{\cosh \tilde{\lambda}} - 1 \right) \\
\tilde{M} = \frac{\tilde{Q} \tanh \tilde{\lambda}}{\tilde{\lambda}} - \tilde{T}_{d} + \left[\frac{1}{\cosh \tilde{\lambda}} - 1 \right] .$$
(6b)

Values of $\overline{\lambda}$, \overline{y} , and \overline{M} for various combinations of \overline{T}_d , \overline{T} and \overline{Q} are given in Table 1.5-2.

TABLE 1.5-1 $\text{VALUES OF $\widetilde{\lambda}$, y and \widetilde{M} FOR SPECIFIED VALUES OF \widetilde{T}_d, \overline{T} and \overline{W} }$

(Pages 1.12 through 1.37)

VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W} o. \overline{v} . o. \overline{v} . \overline{v} .

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VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W}

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VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W}

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TABLE 1.5-1 (Conted) VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W}

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VALUES OF $\bar{\lambda}$, \bar{y} AND \bar{M} FOR SPECIFIED VALUES OF \bar{T}_d , \bar{T} AND \bar{W}

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TABLE 1.5-1 (Cont*d) VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W}

$\overline{\mathfrak{r}}_{_{\mathbf{D}}}$.	0.80000008 81	w ·	0.180000CE 02	T _O	0.80000001 01	w ·	0.71000061 02
ĩ	Ÿ	<u>M</u> .	<u> </u>	T	v	M	- -
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VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W}

$\overline{\tau}_{_{D}}$.	0.12000006 02	w .	p. 3000000£ 3:	ř _p .	0.12 1008 92	w·	0.60000001
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TABLE 1.5-1 (Cont'd) VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W}

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TABLE 1.5-1 (Cont'd) VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W}

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VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W}

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TABLE 1.5-1 (Cont[†]d) VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{W}

$\overline{r}_{\rm p}$.	-0.2000000€ 02	w	0.1830000F	02	$\overline{r}_{\mathfrak{b}}$.	-c.2000000E C2	w ·	3.7100000€ 3.º
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	COMPRESSIVE	LAD LOADS				COMPRESSIVE	END LEADS	
0.2408190E 03 0.2657794E 03 0.2657794E 03 0.505207F 03 0.5377020E 03 0.5855117E 03 0.5855117F 03 0.5468900Q 05 0.594570C 05 0.69450Q 05 0.9446Q71E 03 0.1418202E 04 0.2568195E 04 0.1119358E 05 0.5157945E 05 0.5557524E 06	U.1375000 07 U.13751707 07 U.13737708 07 U.15737708 07 U.15737708 07 U.15737708 07 U.1677100 07 U.27737001 07 U.27737001 07 U.27737001 07 U.4773701 07 U.477370	0.90000301 01 0.10285551 02 0.11355891 02 0.12435841 02 0.12435841 02 0.17455611 02 0.27451140 12 0.3751140 02 0.4751140 12 0.4751140 12 0.4751140 12 0.4751140 12 0.4751140 12 0.4751140 12 0.4751140 12 0.4751140 12 0.4751140 12 0.4751140 14 0.17745711 04	o, r. to tanget exceeded of the control of the cont	00 00 00 00 00 01 01 01 01 01 01	0.76913/34 03 0.78976736 05 0.3475074 03 3.47287772 03 0.4897674 03 0.4897674 03 0.772087 03 0.772087 03 0.134,795 04 0.15477972 04 0.1647781 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05 0.4897774 05	0.16/ 061 02 0.16.7 441 02 0.16.774 12 0.16.7744 12 0.16.7744 16 0.18.8024 12 0.1801274 12 0.1801274 12 0.185104 12 0.1851 14 12 0.1	0.1050334 C2 0.1144744 02 0.1144744 02 0.1202040 02 0.1202040 02 0.1557574 02 0.155	9. a. tonbooke (u.) a.
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TABLE 1.5-2

values of $\bar{\lambda},\,\bar{y}$ and \bar{M} for specified values of $\bar{T}_{\underline{d}},\,\,\bar{T}$ and \bar{Q}

(Pages 1.38 through 1.55)

\overline{r}_{D} ,	0.	ō.	0.	$\vec{\tau}_{_{\mathbf{D}}}$	0.	\overline{q} .	0.30000001 61
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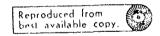
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TABLE 1.5-2 (Cont'd) VALUES OF $\overline{\lambda}$, \overline{y} AND \overline{M} FOR SPECIFIED VALUES OF \overline{T}_d , \overline{T} AND \overline{Q}

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Table 1.5-2 (Cont'd) values of \overline{T}_d , \overline{T} and \overline{Q}

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0-25253820 03	0.11000000 02	0.12003000 02	0.	0-20015530 01	d. 1500000£ 02	C.1500000€ 02	0.
C.27102070 Q3	2.1453869E 02	3.13168≈et 0?	-0.50CCCCCE-C3	0. 11648176 03	0.1557603E 0Z	0.16401846 02	-0.30000000-00
0.28359008 13	0.1499476. 02	0,14497886 02	-0.40000000000	0.32953806 05	0.16055716 02	0.17568536 07 0.19178956 02	-8.4000000£-00 -8.5000000£-00
C. 317.5850t O3 O. 5457037t 05	G. 1560455E 02 C. 1655134E 02	0.15901146 02	-0.50000000 00 -0.50000000 00	0.3567289t 03 0.3948065E 03	0.16/1589E 02	0.21335006 02	-0.40000000 GC
0.57244526 05	C. 17527586 C2	C. 2058891E 32	-0.7008CC0£ C3	0.448/4516 03	0.16771905 02	0.24198226 02	-0.7000000t 00
0.4.45686 0;	0.10984056 02	0.7-151066 02	-0.50000001 00	0.52461746 05	0.20+3157€ 02	0.28012216 02	-0.0800000± 00
0.55601136 05	0.20059800 02	a. 2497663C 02	-0.900000ut 0.	0.43744582 03	0.22440936 02	0.33177156 02	-0.90000000 00
0.71142865 69	0.23705716 02	0.55705218 02	-0.10000306 01	6.01278785 03	0.25377436 02	0.40377438 02	-0.10000000€ 01
0. 4682 CATE 05	2-26707306 02	0.45525836 02	-C.1100000L 01	0.11063615 04	0.29656426 02	0.50834266 02	-0.11000006 01
0. 14694671 05	0. 31967166 02	C. GLY 1558[07	-C.120000UE U1	0.1556322t 04	0.3635136E 07	0.47345964 02	-0.1200000t 01
0.25502028 04	0.44980196 02	0.88016845 02	-0.13000002 61	0.28915601 04	912391E 02	0.00329406 02	-0.1500000E 01
0.57366666 04	0.09044258 02	0.1475659(03	-0.1%00000£ 01	0.6785062E 04	31244E 02	0.15979008 03	-0.14000000 01
C-1147687E 05	C. 98146776 CZ	0.21414866 03	-0.1%50C0GL C1	0.13117286 05	0.10282666 03	0.2311929F 03	-0.16300001 01
0.25354536 02	0.13157696 03	0.3755480(03	-0.1500000£ 01	U. 5445 TOOE OS	0.17277820 03	0.4037509E 03	-0.15000000 0:
0.3627479(06	0.54195478 03	0.14137788 04	-1.1350000E 01	G. 41459341 06	0.57941966 03	0.1607053t #4	-0.1550000€ 01
	TE8511C +	NU LOAOS			TEMSILE E	40 L0407 GA	
0.23515426 03	0.13479108 02	9.10785068 02	0.30000006-00	0.26847361 05	0.14464576 02	0.13693191 02	0.10000001-00
C-2278527E 03	0.13133648 02	3.93786165 01	0.40000006-00	0-25461496 03	0.15073536 02	0.12748236 02	0.40000001-00
G. 20540921 05	C. 1269174E C2	0.88271390 01	0.50000000 60	0.23791286 03	0.13600% SE 02	0.11593898 02	0.5000COUF DG
C. 1975917t Q3	0.17188471 C2	0.76120056 01	0.40000000 00	0.21983901 03	0.1306319E C2	0.1029775t 07	0.8000000k 00
C. 17617196 03	0.11643476 02	0.6296730[0]	0. 1000000£ 00	0.20102676 03	0.12479901 02	0.01848476 01	0.70000000 00
0.15962206 03	0.11071025 52	0.4914550E 01	0.80000000 06	0.1821657£ 03	0.11667686 07	0.74046688 01	0.80000000 00
6.143585At 05	0.10485766 02	0.35005316 01	0.70CC0Q0L C7	0.16384758 03	0.112%17%E 02	0.58941910 01	0.70000000 00
0.12837701 03	0.95927852 01	0.2:002:50 01	0.1000000L 01	C. 1464675E 03	0.1061500£ 02	0.4384998(01	0.100000GL 0:
G. 11471546 03	0.4522812(G1	0.71757776 00	0.1100000 01	G. 13C296%L 03	C.9997870E C1	0.22025776 01	0.110000Ct 01
0.10124438 03	0.07625156 6"	-J.6177507E 00	C.1200000E 01	C-1154811E 03	0.91981298 01	0.14664766 01	0.12000000 01
0.3950430: 02	0.82737315 61	-0.19980576 01	C.13C0000. 01	0.10207376 03	0.88221696 01	0.9053450[-0]	0.13000066 01
0-78980341 07	U. 17106286 C:	-0.31123311 0:	0.14000004 01	C.9005689E C2	0.82732706 (1	-0.12156491 01	0.14000000£ 31
0.69616531 02	0.72252866 11	-0.4256893E 01	0.1500000f 01 0.1600000f 01	0.79366491 62	0.77540436 01	-0.7%%45978 01	0.15000000 03
0.6133079t N2 0.5%029%0t 02	0.67687208 01	-0.5327722E 01 -0.6325765E 01	0.17000001.01	0.69409506 02	0.7265544E 01 0.680* 'IE 01	-0.3599793E 01 -0.4675043E 01	0.1600000E 01
C.4/61518t 02	0.59417338 01	-0.7251867[61	0.130J0GGE UI	0.54258691 02	0.83800 21 01	-0.56738528 01	C.1800000t 01
0.41204546 02	0.55702776 01	-0.81087718 01	0.19C2COUE 01	0.47860201 02	0.59830814 01	-0.65989226 01	0.19000006 01
0.37050936 02	0.52249476 01	-0.4479/705 (1	0.200000000 51	0.42216651 02	0.56136178 01	-0.74537496 01	0.20000000 01
0.3/72675t 02	0.40044400 01	-0.96786792 01	9.21000000 61	0.37290300 02	0.5270365E 01	-0.82423106 01	0.2:000000 21
0.28935136 02	0.50071111 01	-0.10279391 32	0.2200000L 01	0.32972896 02	0.47527308 01	-0.996883%[0'	0.27700002 01
0-25004501 02	0.4331749(31	-0.1071601E 07	0.23000000 01	C. 29185261 02	0.46573966 31	-0.96376236 01	0.2300000E u1
G-2260000E 02	0.40769298 07	-0.11402536 02	0.24000006 01	0.25657498 02	0.53841706 01	-0.10232718 07	0.24000000 01
0.20102166 32	0.384C452E #1	-C-120028 % 07	0.750000CL 01	0.279290at 02	0.41310226 01	-0.10918896 02	0.2500000E 01
0.17324574 07	C. 4621393L 01	-0.1246061L 02	C.2600000k 01	C.2034608E 02	0. 3894 565E C1	-0.11339956 02	0.2000064 01
6.1501218C J2	0. 16187971 01	-0.1719376 02	0.27000000 01	C. 18067501 C2	0.5678775£ C)	-0.11314271 02	0.27000000 01
0.1h02457E 07	C.3229501E 01	-0.13.27428 02	0.2800030£ 01	0.14638291 02	U. \$674890E 01	-0.12258890 02	0.28000000 01
U.1243332E 02	0.40550256 01	·9.1369276E 02	0.2000066 01	C. 14230976 02	0.32494408 01	-0.12664558 02	0.27000000 01
0.11012354 02	0.2892550E 01	-0.1403377£ 02	0.10000004 61	0.12434458 07	0.41154576 C1	-0.15043171 07	0.30000001 01



1.5 (Cont'd)

CASE C - Transverse load uniformly distributed or concentrated at midspan - specified axial loads (zero axial end restraint);

The beam is shown schematically in Figure 1.5-3. Since the ends are unrestrained axially, the beam ends are free to move due to the action of temperature, transverse loads and specified end loads. Hence the compatibility Eqs. (2) and (5) do not apply, and since λ is now a known quantity, the maximum deflections and bending moments may be determined directly from Eqs. (3) and (6). These quantities are independent of the average temperature in the beam.

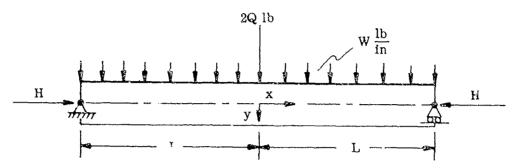
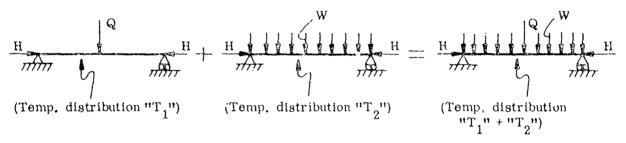
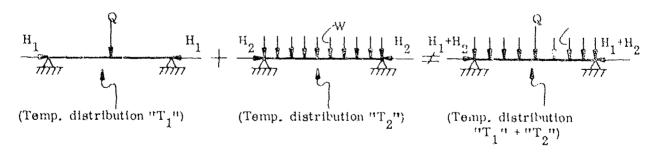


FIGURE 1.5-3. RECTANGULAR BEAM WITH ZERO AXIAL END RESTRAINT AND SPECIFIED AXIAL LOADS

It is important to note that when the end loadings are specified then a modified principle of superposition as shown by Figure 1.5-4a may be used. The figure shows that the resultant effect of several transverse loads and temperature distributions acting simultaneously



(a) Superposition valid for an axially unrestrained beam with specified end loads "H"



(b) Superposition not valid for an axially restrained beam

FIGURE 1.5-4 MODIFIED SUPERPOSITION PRINCIPLE

1.5 (Cont'd)

in the presence of a specified axial load can be obtained by superposing the effects of the individual transverse loads and temperatures acting with the axial load.

However, this superposition principle is <u>not</u> valid when the ends are restrained axially (e.g., Cases A and B). In such cases the axial loads depend on the transverse loads and temperatures nonlinearly.

1.6 USE OF THE TABLES

Tables 1.5-1 and 1.5-2 are reproductions of IBM 7090 digital computer numerical solutions for the beam-column problem. Table 1.5-1 presents results for the case of uniform transverse loading and Table 1.5-2 applies for concentrated midspan loads. Each table is first subdivided into sections corresponding to given temperature differences and transverse loads and then further subdivided into compressive and tensile end loading cases. Since the quantities \overline{W} , \overline{T}_d , and \overline{T} specified in a given problem will not in general coincide with those

listed in the tables, interpolation must be employed. The important quantity to be evaluated is the nondimensional end loading parameter, $\overline{\lambda}$ and the spacing of the tabulated values has been made sufficiently close so as to allow reasonably accurate interpolation for engineering purposes. A systematic interpolation formula will be given as part of an illustrative problem in Sub-section 1.7.

Numerical values are given in terms of a floating decimal number system and are to be interpreted as shown by the following examples

$$0.2456582E \ 00 = 0.2456582 \times 10^{0} = 0.2456582$$

$$0.2456582E 02 = 0.2456582 \times 10^2 = 24.56582$$

$$0.2456582E-01 = 0.2456582 \times 10^{-1} = 0.02456582$$

1.7 NUMERICAL EXAMPLES

EXAMPLE I - Beam with Full Axial End Restraint:

Figure 1.7-1 shows a simply supported strip with immovable ends subjected to a uniformly distributed load of $2\frac{lb}{in}$. The temperature varies linearly through the thickness from 100°F at the upper face to 150°F at the lower face. Young's modulus are the linear coefficient of thermal expansion are taken to be

$$E = 30 \times 10^6 \text{ psi}$$

$$\alpha = 6 \times 10^{-6}$$
 in/in - °F.

Find the axial end load, midspan deflection, bending moment and the maximum stress.

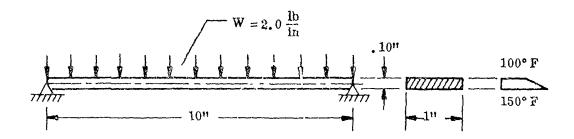


FIGURE 1.7-1 ILLUSTRATIVE PROBLEM; STRIP WITH IMMOVABLE ENDS

SOLUTION

From Figure 1.7-1 and the given data:

$$b = 1^{\circ}$$

$$h = 0.10^{\circ}$$

$$L = 5^{\circ}$$

$$W = 2.0 \frac{1b}{in}$$

$$T_{0} = 100^{\circ} F$$

$$T_{1} = 150^{\circ} F$$

$$E = 30 \times 10^{6} \text{ psi}$$

$$\alpha = 6 \times 10^{-6} \text{ in/in - °F}$$

Therefore

$$\overline{W} = \frac{12W}{Eb} \left(\frac{L}{h}\right)^{4} = \frac{12(2.0)}{(30)(10)^{6}(1)} \left(\frac{5}{0.10}\right)^{4} = 5.0$$

$$\overline{T}_{d} = \alpha \left(\frac{L}{h}\right)^{2} \left(T_{o} - T_{i}\right) = 6(10)^{-6} \left(\frac{5}{0.10}\right)^{2} (100-150) = -0.75$$

$$\overline{T} = \alpha \left(\frac{L}{h}\right)^{2} \left(T_{o} + T_{i}\right) = 6(10)^{-6} \left(\frac{5}{0.10}\right)^{2} (100+150) = 3.75$$

Using the above nondimensional quantities we must now utilize Table 1.5-1 to determine the nondimensional axial loading parameter $\bar{\lambda}$. Since the table does not give $\bar{\lambda}$ directly for the above combination of quantities the following interpolation procedure is recommended:

(1) Determine the next lower and higher values of \overline{W} that are given in the table. Designate these values as \overline{W}_0 and \overline{W}_1 respectively. For this example

$$\overline{W}_0 = 3.0$$
 and $\overline{W}_1 = 6.0$.

1.7 (Cont'd)

(2) Determine the next lower and higher values of \overline{T}_d that are given in the table. Designate these values as \overline{T}_{d0} and \overline{T}_{d1} , respectively.

For this cample

$$\overline{T}_{d0} = -4.0$$
 and $\overline{T}_{d1} = 0$.

(3) For each of the four combinations $(\overline{W}_0, \overline{T}_{d0})$, $(\overline{W}_0, \overline{T}_{d1})$, $(\overline{W}_1, \overline{T}_{d0})$, $(\overline{W}_1, \overline{T}_{d1})$ the table lists $\overline{\lambda}^i$ s corresponding to \overline{T}^i s on either side of the given value for \overline{T} . Denote the values of \overline{T} listed in the tables for the combination $(\overline{W}_1, \overline{T}_{dj})$ by $(\overline{T}_{ij}, \overline{T}_{ij}^i)$ and the corresponding values of $\overline{\lambda}$ by $(\overline{\lambda}_{ij}, \overline{\lambda}_{ij}^i)$ where $\overline{T}_{ij} < \overline{T} < \overline{T}_{ij}^i$

For this example

$$\overline{T}_{00} = 3.397 \qquad \overline{\lambda}_{00} = 1.2
\overline{T}_{00} = 3.896 \qquad \overline{\lambda}_{00}^{\dagger} = 1.1
\overline{T}_{01} = 3.041 \qquad \overline{\lambda}_{01} = -1.2
\overline{T}_{01}^{\dagger} = 5.174 \qquad \overline{\lambda}_{01}^{\dagger} = -1.3
\overline{T}_{10} = 3.416 \qquad \overline{\lambda}_{10} = 1.5
\overline{T}_{10}^{\dagger} = 3.969 \qquad \overline{\lambda}_{10}^{\dagger} = 1.4
\overline{T}_{11} = 3.648 \qquad \overline{\lambda}_{11} = -.8
\overline{T}_{11}^{\dagger} = 4.441 \qquad \overline{\lambda}_{11}^{\dagger} = -.9$$

(4) The value of $\bar{\lambda}$ may now be obtained from the following interpolation for mula.

$$\widetilde{\lambda} = \left[\frac{\widetilde{W} - \widetilde{W}_0}{\widetilde{W}_1 - \widetilde{W}_0} \right] \Lambda + \left[\frac{\widetilde{W}_1 - \widetilde{W}}{\widetilde{W}_1 - \widetilde{W}_0} \right] B$$

where

$$A = \begin{bmatrix} \overline{T}_{d} - \overline{T}_{d0} \\ \overline{T}_{d1} - \overline{T}_{d0} \end{bmatrix} \begin{bmatrix} \left(\overline{T} - \overline{T}_{11} \right) \overline{\lambda}_{11}^{\dagger} + \left(\overline{T}_{11}^{\dagger} - \overline{T} \right) \overline{\lambda}_{11} \end{bmatrix}$$

$$+ \begin{bmatrix} \overline{T}_{d1} - \overline{T}_{d} \\ \overline{T}_{d1} - \overline{T}_{d0} \end{bmatrix} \begin{bmatrix} \left(\overline{T} - \overline{T}_{10} \right) \overline{\lambda}_{10}^{\dagger} + \left(\overline{T}_{10}^{\dagger} - \overline{T} \right) \overline{\lambda}_{10} \end{bmatrix}$$

$$B = \begin{bmatrix} \overline{T}_{d} - \overline{T}_{d0} \\ \overline{T}_{d1} - \overline{T}_{d0} \end{bmatrix} \begin{bmatrix} \left(\overline{T} - \overline{T}_{01} \right) \overline{\lambda}_{01}^{\dagger} + \left(\overline{T}_{01}^{\dagger} - \overline{T} \right) \overline{\lambda}_{01} \\ \overline{T}_{01}^{\dagger} - \overline{T}_{01} \end{bmatrix}$$

$$+ \begin{bmatrix} \overline{T}_{d1} - \overline{T}_{d} \\ \overline{T}_{d1} - \overline{T}_{d0} \end{bmatrix} \begin{bmatrix} \left(\overline{T} - \overline{T}_{00} \right) \overline{\lambda}_{00}^{\dagger} + \left(\overline{T}_{00}^{\dagger} - \overline{T} \right) \overline{\lambda}_{00} \\ \overline{T}_{00}^{\dagger} - \overline{T}_{00} \end{bmatrix}$$

Substituting the known quantities into the above formula yields

$$\bar{\lambda} = -.524$$

Thus the axial end load is given by

$$H = \frac{E\bar{\lambda}^2}{L^2} = \frac{(30 (10)^6 [(1) (.10)^3 / 12] (-.524)^2}{(5)^2} = 27.4 \text{ lb. (compression)}$$

The nondimensional deflection and bending moment at midspan can be determined from Table 1.5-1 using an identical interpolation procedure. However, a simpler and more accurate method is to substitute the known values of $\overline{\lambda}$, \overline{W} , and \overline{T}_d into Eqs. (3a) of Subsection

1.5*). Direct substitution yields

$$\overline{y} = \begin{bmatrix} \underline{y} \\ h \end{bmatrix}_{X=0} = 1.60 \text{ and}$$

$$\overline{M} = \begin{bmatrix} \frac{12ML^2}{Ebh^4} \end{bmatrix}_{X=0} = 2.94 ,$$

so that

$$y \Big]_{X=0} = h \overline{y}$$
= (.10) (1.60) = .16 in. (downward)
$$M \Big]_{X=0} = \frac{Ebh^{\frac{4}{3}}}{12L^{2}} \overline{M}$$
= $\frac{30 (10)^{6} (1) (.10)^{\frac{4}{3}}}{12 (5)^{2}}$ (2.94)
= 29.4 in.-lb (compression in upper fiber)

^{*} Eqs. (3b) are not to be used here since they apply only for $\tilde{\lambda} > 0$ (tension)

1.7 (Cont'd)

The maximum stress is thus

$$\sigma_{\text{max}} = \frac{H}{bh} + \frac{M(6)}{bh^2} = 17,900 \text{ psi.}$$

The effect of axially restraining the beam is evidenced by comparing these results with those for an axially unrestrained and simply supported beam subjected to the same transverse loading and temperature but with H=0. In such a case the central deflection, bending moment and maximum stress are given by (Section 4 of Reference 1-2):

$$y \Big]_{x=0} = \frac{5W(2L)^4}{384EI} + \frac{\alpha (T_i - T_0) L^2}{2h}$$

$$= .104 + .038$$

$$= .142 in$$

$$M \Big]_{x=0} = \frac{W(2L)^2}{8} = 25 in-lb,$$

$$\sigma_{max} = 15,000 psi.$$

Thus, in this case, neglecting axial end restraint which may be present yields unconservative results for both the central deflection and maximum stress.

EXAMPLE II - Unrestrained Beam Column With Prescribed Axial Load:

Figure 1,7-2 shows a simply supported strip with movable ends and a prescribed 20 lb tensile load subjected to a concentrated midspan load of 10 lbs. The temperature varies linearly through the thickness from 200°F at the upper face to 150°F at the bottom face. Young's modulus and the linear coefficient of thermal expansion are given as

$$E = 30 \times 10^{6} \text{ psi}$$

 $\alpha = 6 \times 10^{-6} \text{in/in} - {}^{\circ}\text{F}$

Find the midspan deflection, bending moment and the maximum stress.

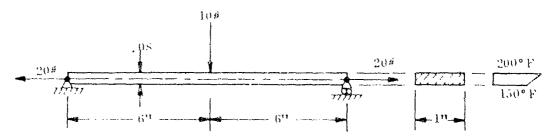


FIGURE 1,7-2 ULLUSTRATIVE PROBLEM; STRIP WITH MOVABLE ENDS

1.7 (Cont*d)

SOLUTION

From Figure 1.7-2 and the given data:

$$b = 1$$
"

 $T_0 = 200^{\circ} F$
 $h = .08$ "

 $T_1 = 150^{\circ} F$
 $L = 6$ "

 $Q = 5 \text{ lb}$
 $T_1 = 20 \text{ lb}$
 $T_1 = 150^{\circ} F$
 $T_2 = 150^{\circ} F$
 $T_3 = 150^{\circ} F$
 $T_4 = 150^{\circ} F$
 $T_5 = 30 \times 10^6 \text{ psi}$
 $T_5 = 30 \times 10^6 \text{ psi}$
 $T_5 = 30 \times 10^6 \text{ psi}$

Therefore

$$\bar{Q} = \frac{12QL^3}{Ebh^4} = \frac{(12)(5)(6)^3}{30(10)^6(1)(.08)^4} = 10.55$$

$$\bar{T}_d = \alpha \left(\frac{L}{h}\right)^2 - (T_0 - T_1) = 6(10)^{-6} \left(\frac{6}{.08}\right)^2 - (200 - 150) = 1.69$$

$$\bar{\lambda} = \sqrt{\frac{HL^2}{EI}} = \sqrt{\frac{20(6)^2}{(30)(10)^6(1)(.08)^3/12}} = .75$$

As discussed in Sub-section 1.5, since the ends are free to move, the bending response of the beam is independent of the average temperature. The central deflection* and bending moment can thus be obtained by direct substitution of the nondimensional parameters into Eqs. (6b) of Sub-section 1.5. This yields

$$\overline{y} = \begin{bmatrix} \underline{y} \\ \overline{h} \end{bmatrix}_{X = 0} = .674$$

$$M = \begin{bmatrix} \frac{12ML^2}{Ebh^4} \end{bmatrix}_{X = 0} = 2.94$$

so that

y
$$\int_{|X|=0}$$
 = .054 in (downward)

^{*} This is not always the maximum deflection (see Sub-section 1.8)

1.8 ADDITIONAL CONSIDERATIONS

(1) Application of Basic Equations to the Case of Unequal Elastic End Restraints.

The general formulas presented in Sub-section 1.3 have been developed for the case of equal elastic end restraints of stiffness 2K (Figure 1.3-1). As is frequently the case, these restraints may be unequal. This situation can be accommodated by replacing the quantity 2K in Sub-section 1.3 with the equivalent quantity $\frac{4K_1K_2}{K_1+K_2}$ where $2K_1$ and $2K_2$ are the unequal spring stiffnesses. Thus for example in the special case where one end is held $(K_1=\infty)$ then $\lim_{K_1\to\infty}\frac{4K_1K_2}{K_1+K_2}=4K_2; \text{ and this quantity is to be substituted for } 2K \text{ in Eqs. (3) and (4) of Sub-section 1.4.}$ This procedure will yield the correct results for the deflection and bending moment.

(2) Maximum Bending Moments and Deflections.

This report presents results for the determination of the central deflection and bending moment for a symmetrically loaded beam-column. It can be shown by the usual maximum-minimum procedure that the central bending moment is always numerically the largest bending moment in the beam. If the temperatures and loads tend to produce curvatures in the same direction then the central deflection is a maximum. However where temperature and loads tend to relieve each other, the central deflection may not be a maximum. If this quantity is desired it may be found by considering the full deflection formula (Eq. (1) of Sub-section 1.4).

1.9 REFERENCES

- 1-1 Timoshenko, S., "Theory of Elastic Stability," McGraw Hill Book Company, Inc. pp. 6-8, 1936.
- Switzky, H., Forray, M., and Newman, M., "Thermo-Structural Analysis Manual"-Volume I, Sections 1, 2, and 4 of Republic Aviation Corporation Report No. RAC 679-1, September 1960, revised November 1961 (to be published as WADD TR 60-517, Vol. I).

SECTION 2

APPROXIMATE SOLUTION FOR AN AXIALLY RESTRAINED COLUMN SUBJECTED TO ELEVATED TEMPERATURES AND LATERAL LOADS

bу

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2.0



SECTION 2

APPROXIMATE SOLUTION FOR AN AXIALLY RESTRAINED COLUMN SUBJECTED TO ELEVATED TEMPERATURES AND LATERAL LOADS

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SECTION 2

APPROXIMATE SOLUTION FOR AN AXIALLY RESTRAINED COLUMN

SUBJECTED TO ELEVATED TEMPERATURES AND LATERAL LOADS

2.1 SUMMARY

Approximate solutions are obtained for the axial compressive load and the lateral deflections of columns which are restrained by an axial spring and subjected to an increase in temperature. The solutions are presented graphically in terms of nondimensional parameters.

The problem considered is a column of arbitrary cross section with pinned or clamped ends subjected to an arbitrary temperature and lateral load distribution in addition to flaite initial eccentricities. The effects of the thermal gradients, lateral loads, non-linear axial springs, and plasticity of the material are discussed. The simpler problem of constant bending stiffness is explored to illustrate the evaluation of the nondimensional parameters.

2.1.1 Definition of Symbols

The following symbols are used throughout this section:

	Associated of lateral defloration lands
a v	Amplitude of lateral deflection, inches
ь	Axial extension parameter (b = $\alpha T/\epsilon_1$)
b	Width of rectangular cross section, inches
d	Axial shortening parameter $\left(1_{i} = \frac{\Delta_{j}}{\epsilon_{1}}\right)$
ħ	Depth of cross section of column, inches
i	Integer indicating order of the deformation mode
k	Axial stiffness of end restraint, pounds per inch
1	Longth of column, inches
$_{\rm j}$	Coefficients expressing the lateral deformation as a polynomial, inches
q	Lateral lead, pounds per inch
r	Force function. Ratio of axial load in column to a reference load $(r(x) = F(x) - F_0)$
W	Lateral deflection of column, inches
X	Axial coordinate of column, inches
Z	Lateral coordinate of column cross section, makes
Č.	Coefficient of thermal (thear) expansion, inches per inch. *F
Δ	Deflection of ends of column, inches
Δ	Incremental change
€	Axial strain due to axial load or temperature, inches per inch
$\epsilon_{\rm i}$	Buckling strain corresponding to $i^{ ext{th}}$ mode $\left(\!\epsilon_{i}^{-} \! = \! \lambda_{i}^{-} \! o^{2}^{-} \! ight)$
*;	Load parameter $\left(\begin{array}{cc} n_1 & \frac{C}{F_1} \end{array}\right)$
ж	Curvature of column, 1 inches
$\lambda_{\mathbf{i}}$	Eigenvalue for which non-trivial solutions of the differential equilibrium
1	·
	equation exist $\left(\lambda_i = \frac{F_i}{EI}\right)$, $1/sq.$ inches

2.1.1 (Cont'd)

```
Eigenvalue (\mu_i - \tan \mu_i)
                     Nondimensional axial coordinate (\xi = x/\ell)
ξ
                     Effective radius of gyration = \sqrt{\overline{E}} \overline{I}/\overline{E}\overline{A}, inches
ρ
                     Axial stress (\sigma = F/A), psi
σ
                     Stiffness function. Ratio of bending stiffness of cross section to a reference
0
                       bending stiffness \left(c(x) = \frac{EI(x)}{EI}\right)
                     Additional axial shortening function \left(\varphi_{i} = \left(\frac{1}{1-\eta_{i}}\right)^{2} - 1\right)
                     Nondimensional linear axial shortening term \left(\Phi = 1 + \frac{EA}{k\ell} + 2\sum_{i} \frac{\eta_i d_i}{\eta_i}\right)
Φ
                     Cross sectional area, sq. inches
A
C
                     Amplitude of curvature (x = \Sigma C_i \chi)
                     Secant modulus (E_S = \sigma/\epsilon i for a linear material), psi Axial stiffness (\overline{EA} = \int_S E_S \, dA, note \int_S E_S \, z \, dA = 0), lo
E_s
EA
                     Bending stiffness (\overline{\rm EI} = \frac{1}{2} E_{\rm S} z^2 dA), pounds square inches
\mathbf{F}\mathbf{I}
Ι.,
                     Axial load in column, 1b
                     Moment of inertia of cross sections, inches
I
                     Haif length of column (1, = 2), inches
l.
М
                     Moment (M = EI w^n), pounds per inch
Þ
                     Redundant transverse lead, ib
T
                     Temperature increment from datum, "F
                                                     SUBSCRIPTS
                     Condition before application of axial load
Ø
                     th
i mode
į
                     Due to lateral mechanical leads
Q.
Ţ,
                     Due to temperature
                     Simple simple boundary conditions
8.8.
                     Simple clamped no Tary conditions
S. C.
                     Clamped clamped is arrany conditions
c.c.
                     Symmetrical mode
3
                     Anti-symmetrical mode
```

2, 1, 2 GENERAL APPROACH

The analysis of the problem is approached by examining the determation characteristics of the column with a linear material to obtain a solution which sat, f(s,s) impatibility and equilibrium. The following steps are employed to solve the column shown in Figure 2.4.24.

(1) The column deforms in characteristic modes which are dependent upon the distribution of the 'ending stiffness and axial load in the column as well as the boundary (end) con

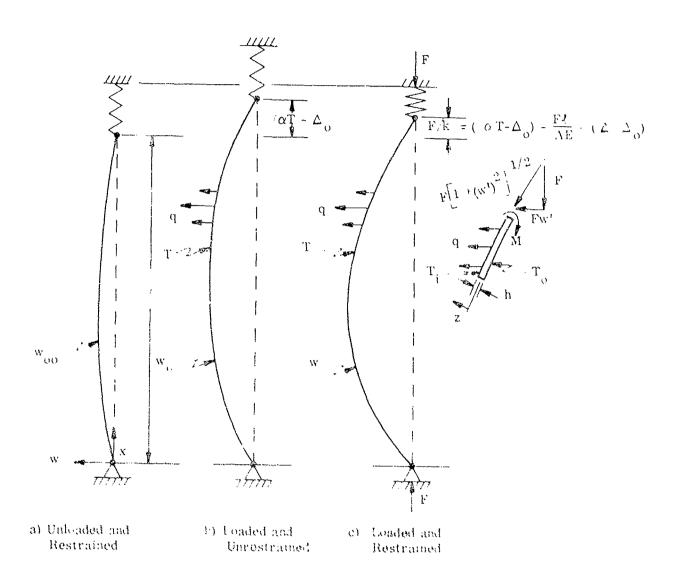


FIGURE 2.1, 2-1 RESTRAINED COLUMN

2.1.2 (Cont'd)

or clamped ends.

ditions. Basic orthogonality relationships exist among the characteristic deformation modes which are used in obtaining a solution for the column in a thermal and mechanical environment.

- (2) The deformations of a column subjected to lateral loadings and temperature gradients in addition to an axial load are then determined by solving the equilibrium equation. The analysis results in a solution in terms of the "initial" deformations caused by the lateral loads, thermal gradients, and manufacturing and loading eccentricities acting on a column with no axial load. These "initial" deformations grow with the magnitude of the axial load. Each characteristic deformation mode is magnified as a rectangular hyperbola, by a factor $\left(\begin{array}{c} 1 \\ \hline 1-\tau. \end{array}\right)$ which is a function of the applied load and the characteristic "buckling" load corresponding to the deformation mode. The method of determining deformation modes and corresponding buckling loads is illustrated for columns of constant bending stiffness with pinned
- (3) The axial load is then appressed as the solution of a compatibility equation which considers the thermal expansion, the lateral deflection, and the axial deformation of a restraining spring. The axial load determined by this method satisfies both the equilibrium and compatibility conditions of the structure. The solution is, therefore, correct for a reversible (one to one relationship of load and deformation) structure even though a column deforms non-linearly with load.
- (4) The compatibility equation is quite difficult to solve but a simple and reasonable approximation of the additional axial shortening of the column due to the axial load reduces the compatibility equation to a form which can be readily solved by graphical means.
- (5) The solutions of the axial load and deformation of columns are presented for pinned (simple) or clamped ends simplified by applicable formulae and graphs together with a computational procedure and illustrative problems. The techniques to obtain the "initial" deformations due to lateral load and thermal gradients, and the effective stiffness of the restraining spring are also presented. The effects of nonlinearity in the spring or material are discussed.

2.2 ANALYSIS

The mathematical relationships necessary to solve the problem are derived in Reference 2-1. The results are summarized below.

2.2.1 Relationships of Deformation Modes

Various orthogonality relationships exist between the characteristic deflection modes and their derivatives whenever the boundary conditions are natural 'a.g., free, clamped, simple, etc.). These relationships are useful in evaluating the initial and final lateral deformations as well as the axial shortening in terms of these characteristic modes.

Orthogonality relationships, (see Section 1A of Reference 2-1) are obtained by solving the homogeneous differential equilibrium equation

$$(\phi \mathbf{w}^{\dagger})^{\dagger} + \lambda (\mathbf{r} \mathbf{w}^{\dagger})^{\dagger} = 0 \tag{1}$$

2.2.1 (Cont'd)

where

 φ is the ratio of bending stiffness to a reference bending stiffness (E $_{0}$ I $_{0}$)

w is the lateral deflection

r is the ratio of the axial load in the cross section to the reference load (F_o)

 λ is the eigenvalue $\frac{F_o}{F_o I_o}$

The following orthogonality relationships are derived

$$\int_{0}^{\ell} (\mathbf{r} \mathbf{w}_{i}^{\prime})^{\dagger} \mathbf{w}_{k} \, d\mathbf{x} = 0 \qquad \qquad i \neq k$$
 (2)

Thus the characteristic modes, which are the solutions to the differential equilibrium equation, are orthogonal to the derivative of the weighted slope $[(r w^i)^i]$. For end loads (r=1), the deflection and curvature modes are orthogonal. This relationship permits the determination of the amplitudes of the deflection modes for the lateral deflections caused by the lateral load and thermal gradients and permits the determination of the growth of these modes when the column is compressed.

$$\int_{0}^{t} rw_{i}^{t} w_{k}^{t} dx = 0 \qquad i \neq k$$
 (3)

Similarly, the slopes of the deflection modes are orthogonal with respect to a load weighting factor (r). This relationship permits the rapid determination of the axial shortening of the column in terms of the magnitude of the deformation modes.

$$\int_{0}^{\ell} c w_{i}^{tt} w_{k}^{tt} dx = 0 \qquad i \neq k$$
 (4)

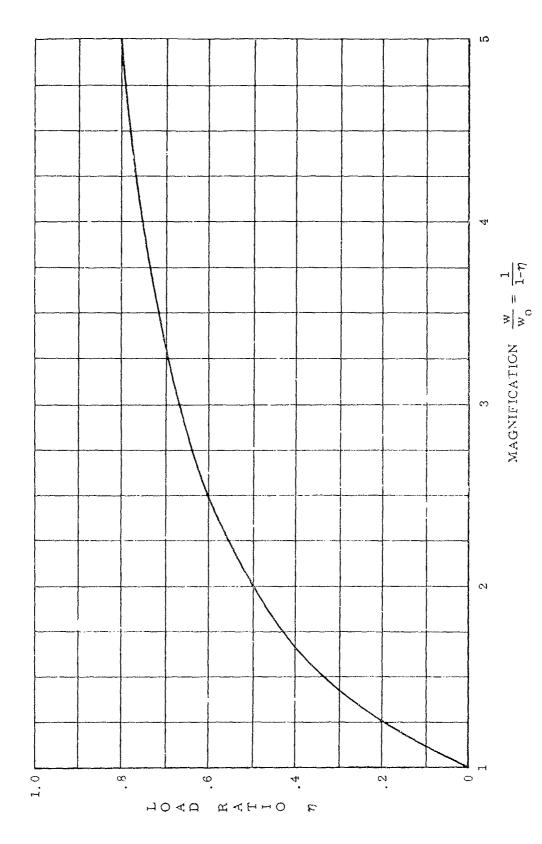
The orthogonality of the curvature with respect to the stiffness weighting function (ω) , permits the solution of the non-homogeneous differential equilibrium equation resulting from the initial deformations due to the lateral load and thermal gradients. The effect of the lateral load and temperature can be expressed as initial curvatures which grow as a rectangular hyperbola with an increase in axial load as indicated in Figure 2.2.1-1.

2.2.2 Solution of Non-Homogeneous Differential Equilibrium Equation

The homogeneous differential equilibrium equation can be rewritten as a function of the curvature $(x = w^{tt})$

i.e.
$$(\varphi x)^m + \lambda r x = 0$$
 (1)

The solutions of this equation results in eigenvectors of the curvature which are orthogonal with respect to the weighting function ϕ (Eq. (4) of Paragraph 2.2.1). It is assumed that these curvature modes form a closed set for the "natural" boundary conditions (columns of constant EI ($\phi = 1$) result in Fourier expansions). Thus, any curvature can be expressed as a weighted sum of the eigenvectors.



MAGNIFICATION OF DEFORMA ION MCDES

FIGURE 2.2.1-1

2.7

where
$$(\omega x_i)^{ij} + \lambda_i r x_i = 0$$
 (2b)

If the unrestrained column is not straight because of initial eccentricities, thermal gradients, and lawral loads, then the moment acting on the column is proportional to the change in curvature and the equilibrium equation becomes, for an end loaded column (r = 1),

$$(\alpha \times)^{\alpha + \lambda} = \left[\varphi \left(x_{\zeta} + x_{\gamma} \right) \right]^{-\alpha}$$
 (3)

where κ_{ij} is the curvature caused by the lateral load and κ_{T} is the curvature caused by the thermal gradient.

Expressing the solution as a weighted sum of the characteristic curvatures (Eq. (2a)) which satisfy the homogeneous equation and manipulating Eq. (3) results in the solution for the curvature of an axially loaded column in terms of the initial curvature and the load ratio.

$$x = \sum_{i=1}^{b_i} \frac{b_i}{1 - n_i} \quad x_i \tag{4}$$

where $b_i \gamma_i$ corresponds to the i^{th} component of the initial curvature of the column due to the lateral load and temperature. Thus the column deforms under axial load by having each component of the initial curvature increase by a magnification factor $1/(i-\eta_i)$. Correspondingly the slopes and lateral deflection modes increase by this same factor.

Since the initial deformations are expressible as an infinite series of the characteristic curvatures, it becomes expeditious to employ the initial curvatures and to integrate this infinite series to obtain the expression for the slope which is employed in obtaining the axial shortening. Employing the initial slope or lateral deflection will result in a slower convergence of the series expressing the axial shortening and would reduce the accuracy of the approximate procedure employed to solve the compatibility equation.

2.2.3 Determination of Deformation Modes and Buckling Loads

The deflection modes (w) of the column are obtained by solving the homogeneous equilibrium Eq. (1) of Paragraph 2.2.1. The buckling loads (F_i) are those values of the axial load which result in non-trivial solutions of the equation. The technique of obtaining these items is illustrated for a column of constant bending stiffness ($\varphi = 1$).

The homogeneous equilibrium equation for constant bending stiffness (EI) is

$$\frac{d^4w}{dx^4} + \frac{F}{E1} \frac{d^2w}{dx^2} = w^{IV} + \lambda w'' = 0$$
 (1)

The general solution is

$$w = c_1 + c_2 x + c_3 \cos \sqrt{\lambda} x + c_4 \sin \sqrt{\lambda} x$$
 (2)

where e1, e2, e3, e4 are constants to be determined by the boundary conditions.

2.2.3 (Cont'd)

Solutions for various boundary conditions can be found in various texts. The simple-simple, clamped-clamped, and simple-clamped are summarized below. In addition the method of determining the magnitude of the initial curvature modes is indicated.

(1) Simple-Simple Column

The boundary conditions are

$$\mathbf{w}(0) = \mathbf{w}(\ell) = 0$$

$$\mathbf{w}^{it}(0) = \mathbf{w}^{it}(\ell) = 0$$

$$\therefore \mathbf{w}_{i} = \mathbf{a}_{i} \sin \frac{i \pi \mathbf{x}}{\ell} = \mathbf{a}_{i} \sin \sqrt{\lambda_{i}} \mathbf{x}$$
 (3a)

where
$$w = \sum w_i$$

$$\frac{\mathbf{F}_{i}}{\mathbf{E}\mathbf{I}} = \lambda_{i} = \left(\frac{\mathbf{i} \cdot \mathbf{\tau}}{\ell}\right)^{2} \tag{4a}$$

$$u_{i} = \sin \frac{i \pi x}{l} \tag{5a}$$

$$\int_0^\ell |\mathbf{x}_i|^2 \, \mathrm{d}\mathbf{x} = \ell/2$$

$$C_{i} = \frac{\int_{0}^{\ell} w^{n} x_{i} dx}{\int_{0}^{\ell} x_{i}^{2} dx} = \frac{2}{\ell} \int_{0}^{\ell} w^{n} \sin \frac{i \pi x}{\ell} dx$$
 (6a)

where $w'' = C_i x_i$

(2) Clamped-Clamped Column

The boundary conditions are

$$w(0) = w(\ell) = 0$$

$$\mathbf{w}^{\dagger}(0) = \mathbf{w}^{\dagger}(0) = 0$$

This results in two different types of modes. Modes which are symmetrical about the mid-length of the column and modes which are anti-symmetrical.

For symmetrical modes

$$w_{is} = a_i \left(1 - \cos \frac{2 i \pi x}{\ell}\right) \tag{3b}$$

2, 2, 3 (Cont'd)

$$\frac{F_{i}}{EI} = \lambda_{i} = \frac{4 i^{2} \pi^{2}}{L^{2}} \tag{4b}$$

$$x_{is} = \cos \frac{2 i \pi x}{\ell} \tag{5b}$$

$$C_{is} = \frac{\int_0^\ell w'' x_i dx}{\int_0^\ell x_i^2 dx} = \frac{2}{\ell} \int_0^\ell w'' \cos \frac{2 i \pi x}{\ell} dx$$
 (6b)

For anti-symmetrical modes

$$w_{i} = a_{i} \left(\frac{2\mu_{i} \frac{x}{\ell} - \sin 2\mu_{i} \frac{x}{\ell}}{2\mu_{i} - \sin 2\mu_{i}} - \frac{1 \cdot \cos 2\mu_{i} \frac{x}{\ell}}{1 - \cos 2\mu_{i}} \right)$$

$$\mathbf{x_i} = \frac{\sin 2\mu_i \frac{\mathbf{x}}{\ell}}{2\mu_i - \sin 2\mu_i} - \frac{\cos 2\mu_i \frac{\mathbf{x}}{\ell}}{1 - \cos 2\mu_i} \quad \text{where } \mu_i = \tan \mu_i$$

An alternate form, employing the mid-length of the comman as an origin, results in a simpler result which is identical to the joining of two simple-clamped columns.

$$w_{ia} = a_i \left(\frac{\sin \frac{\mu_i}{2} - \frac{x}{\ell}}{\frac{\mu_i}{\sin \frac{\mu_i}{2}} - \frac{x}{2\ell}} \right)$$
 (3e)

$$\frac{F_{ia}}{E1} = \lambda_i = \left(\frac{u_1}{4/2}\right)^2 = \frac{4\mu_1^2}{12}$$
 (4e)

where $\tan \mu_i = \mu_i$

and
$$\mu_1 = 1.43$$
 $\mu_n + \left(\frac{2n+1}{2}\right)\pi$

$$x_{ia} = \sin \frac{u_i}{2} \frac{x}{t} \tag{5e}$$

$$C_{ia} = \frac{\int_{0}^{\ell/2} w'' x_{i} dx}{\int_{0}^{\ell/2} x_{i}^{2} dx} = \frac{\ell}{\ell \left(\frac{1}{4} - \frac{\cos \mu_{i}}{\mu_{i}}\right)} \int_{0}^{\ell/2} w'' \sin \frac{\mu_{i}}{2} \frac{x}{\ell} dx$$
 (6e)

2, 2, 3 (Cont'd)

(3) Simple Clamped Column

The boundary conditions are

$$w(0) = w''(0) = 0$$

$$w(\ell) = w^{\dagger}(\ell) = 0$$

$$w_{i} = a_{i} \left(\frac{\sin \mu_{i} \frac{x}{\ell}}{\sin \mu_{i}} - \frac{x}{\ell} \right) \tag{3d}$$

where $\tan \mu_i = \mu_i$

$$\frac{\mathbf{F_i}}{\mathbf{EI}} = \lambda_i = \frac{u_i^2}{\sqrt{2}} \tag{4d}$$

$$x_i = \sin u_i \frac{x}{i} \tag{5d}$$

$$C_{ia} = \frac{\int_{0}^{\ell} w^{n} x_{i} dx}{\int_{0}^{\ell} \frac{2}{x_{i}^{2}} dx} = \frac{1}{\ell \left(\frac{1}{2} - \frac{\cos 2u_{i}}{u_{i}}\right)} \int_{0}^{\ell} w^{n} \sin u_{i} \frac{x}{\tau} dx$$
(6d)

The deformation modes and buckling toads are required in the determination of the end toad and deflection of a restrained column as indicated in Subsection 2.3. If the boundary conditions or stiffness destribution do not conform to the case summarized above, then the buckling loads and modes must be determined for the column considered. Special cases may be found in various text and recourse must be taken to approximate solutions when the column becomes more involved.

The method of solution for a column with a linear material, proposed in this report, is quite general and can handle the more complex configurations provided the initial deflection can be described by characteristic eigenvectors with known eigenvalues.

2, 2, 4 Compatibility Equation

The axially unrestrained column (Figure 2.1-1b) will determ when subjected to lateral loads and temperature. This will cause an axial motion of the ends of the column of $\Delta_{(n)}$.

$$\Delta_{OO} = \{\alpha \text{ T - } \Delta_{O}$$
 (1a)

where $\alpha \, T = \epsilon_T$ is the unrestrained axial strain due to temperature. If the strains are not uniform then

$$\alpha T = \frac{1}{\ell} \int_0^{\ell} \frac{\int \alpha \, T \, dA}{\int dA} \, dx \tag{1b}$$

2, 2, 4 (Cont'd)

 $\Delta_{_{\rm O}}=$ axial shortening of the column due to the lateral load and temperature gradients through the cross sections

$$\Delta_{0} = \frac{1}{2} \int_{0}^{L} (w_{T}^{\dagger} + w_{q}^{\dagger})^{2} dx$$
 (1c)

The application of the axial load causes the column to shorten due to the axial strain $\frac{F\ell}{AE}$ and additional axial shortening $(\Delta-\Delta_0)$. The final movement of the ends must be equal to the deformation of the axial spring F/k.

$$\therefore \ell \alpha T - \Delta_{O} - \frac{F\ell}{AE} - (\Delta - \Delta_{O}) = \frac{F}{k}$$
 (2a)

Dividing by the quantity $\,\ell \, \epsilon_1 \,$ results in

$$\frac{\alpha T - \frac{\Delta_0}{\epsilon_1}}{\epsilon_1} - \frac{F}{AE \epsilon_1} - \frac{\Delta - \frac{\Delta_0}{\epsilon_1}}{\epsilon_1} - \frac{F}{k^T \epsilon_1} = 0$$
 (2b)

where
$$\epsilon_1 = \frac{F_1}{AE} = \frac{\lambda_1 EI}{AE} = \lambda_1 \rho^2$$
 (2e)

Noting that

$$\frac{F}{AE} = \frac{\sigma A}{AE} = \frac{E \epsilon A}{AE} = \frac{\epsilon}{\epsilon_1} = \frac{\epsilon}{\epsilon_1} = \eta_1$$
 (3a)

and

$$\frac{F}{k\ell\epsilon_1} = \frac{E \epsilon A}{k\ell\epsilon_1} = \frac{EA}{k\ell} - n_1 \tag{3b}$$

and letting

$$\frac{\alpha T - \Delta_{O}/\ell}{\epsilon_{1}} = b \tag{4}$$

and evaluating the axial shortening

$$\Delta_{o} = \frac{1}{2} \int_{0}^{\ell} (w_{T}^{\dagger} + w_{q}^{\dagger})^{2} dx = \frac{1}{2} \int_{0}^{\ell} (w_{o}^{\dagger})^{2} dx = \frac{1}{2} \int_{0}^{\ell} (\sum w_{1o}^{\dagger})^{2} dx$$
 (5a)

but because of the orthogonality of the characteristic slopes (Eq. (3) of Paragraph 2, 2, 1) we obtain

$$\Delta_{0} = \frac{1}{2} \int_{0}^{L} \sum (w_{10}')^{2} dx = \frac{1}{2} \sum \int_{0}^{L} (w_{10}')^{2} dx$$
 (5b)

2.2.4 (Cont'd)

and

$$\Delta - \Delta_{o} = \frac{1}{2} \int_{0}^{\ell} \left[(w^{t})^{2} - (w^{t}_{o})^{2} \right] dx = \frac{1}{2} \int_{0}^{\ell} \Sigma (w^{t}_{io})^{2} \left[\left(\frac{1}{1 - n_{1}} \right)^{2} - 1 \right] dx$$

$$= \frac{1}{2} \sum_{i=1}^{\ell} \left[\left(\frac{1}{1 - r_{i}} \right)^{2} - 1 \right] \int_{0}^{\ell} (w^{t}_{io})^{2} dx$$
(5e)

letting

$$\frac{\Delta_{i}}{\ell \epsilon_{1}} = \frac{\frac{1}{2} \int_{0}^{\ell} \left(w_{io}^{\dagger} \right)^{2} dx}{\ell \epsilon_{1}} \qquad d_{i}$$
 (6a)

and

$$\left(\frac{1}{1-\eta_i}\right)^2 -1 = \epsilon_i \tag{6b}$$

we obtain

$$\frac{\Delta_{o}}{\iota \epsilon_{1}} = \Sigma d_{i} \tag{6c}$$

and

$$\frac{\Delta - \Delta_{o}}{t \, \epsilon_{1}} = \sum d_{1} \, \varphi_{1} \tag{6d}$$

Substituting Eqs. (3a), (3b), (4), and (6i) into Eq. (2b) results in

$$\mathbf{b} - \eta_1 - \sum \mathbf{d}_i \, \phi_i \qquad \frac{\mathbf{E}\mathbf{A}}{\mathbf{k}\overline{\mathbf{\ell}}} \quad \eta_1 = 0 \tag{7}$$

The above equation is quite difficult to solve, but the solution can be simplified if it is noted that the expression $d_{i,j}^{c}$ denotes the increase in axial deformation due to the growth of

the i^{th} mode of deformation. The first mode is magnified to a much greater degree than the higher modes when the axial load is compression. If fact the first mode becomes predominant as the axial load approaches the first buckling load. The approximate method utilizes this fact in approximating the value of $\sum d_i \phi_i^{-th}$ expanding $d_i \phi_i^{-t}$ (for $i \geq 1$) as a power series and only employing the most significant term and by a proximating the infinite series by a finite series. This approximation mend is not recommended for tension loads, since the approximation for ϕ_i^{-t} and the infinite series may be incorrect.

2.2.4 (Cont'd)

Noting that

$$\varphi_{\mathbf{i}} = \left(\frac{1}{1-\eta_{\mathbf{i}}}\right)^2 - 1 = 1 + 2\eta_{\mathbf{i}} + 3\eta_{\mathbf{i}}^2 + \dots - 1 \sim 2\eta_{\mathbf{i}}$$
(8a)

and that $1 > \eta_1 > \eta_1$ (i ≥ 2)

$$\therefore \sum_{i=1}^{\infty} d_{i} \varphi_{i} = d_{1} \varphi_{1} + \sum_{i=2}^{\infty} d_{i} \varphi_{i} \text{ is approximately } d_{1} \varphi_{1} + 2 \sum_{i=2}^{n} \eta_{i} d_{i}$$

$$(8c)$$

where n is a sufficiently large integer.

Equation (7) can thus be approximated by

$$b - d_1 \varphi_1 - \left(1 + \frac{EA}{k} + 2 \sum_{i=2}^{n} \frac{\eta_i}{\eta_1} d_i\right) \eta_1 = 0$$
 (9a)

Letting

$$\Phi = 1 + \frac{EA}{kL} + 2 \sum_{i=2}^{n} \frac{\eta_i}{\eta_1} d_i$$
 (9b)

we obtain

$$\frac{b}{\Phi} - \frac{d_1}{\Phi} c_1 - \eta_1 = 0 \tag{9c}$$

or

$$\overline{b} - \overline{d}_1 \sigma_1 - \eta_1 = 0 \tag{9d}$$

where

$$\overline{b} = \frac{b}{\Phi} \tag{9e}$$

and

$$\overline{d_1} = \frac{d_1}{\Phi} \tag{9 f}$$

The value of the axial strain parameter (η_1) can be determined from a graphical plot (Figure 2.2.4-1 and -2) of the variation of \overline{b} with η_1 for various \overline{d}_1 . This value of η_1 can then

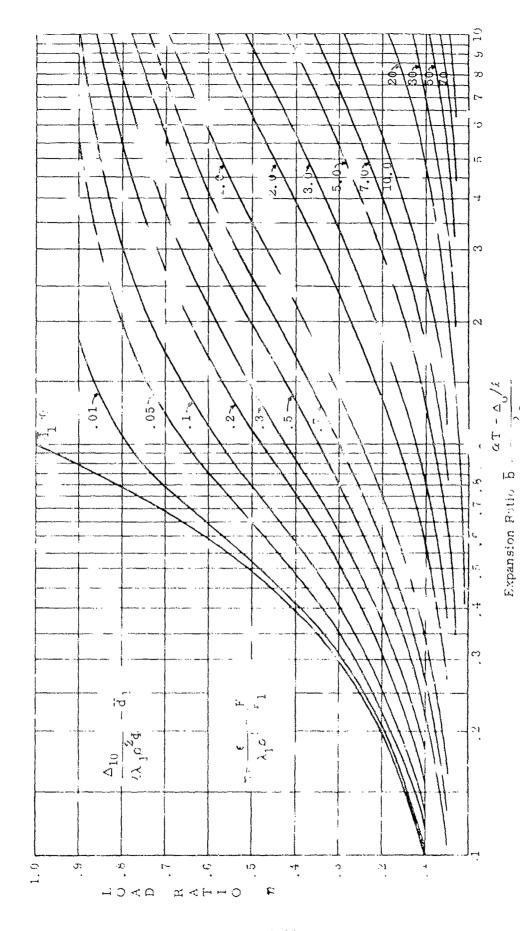


FIGURE 2, 2, 4-1 AXIAL LOAD IN A RESTRAINED COLUMN

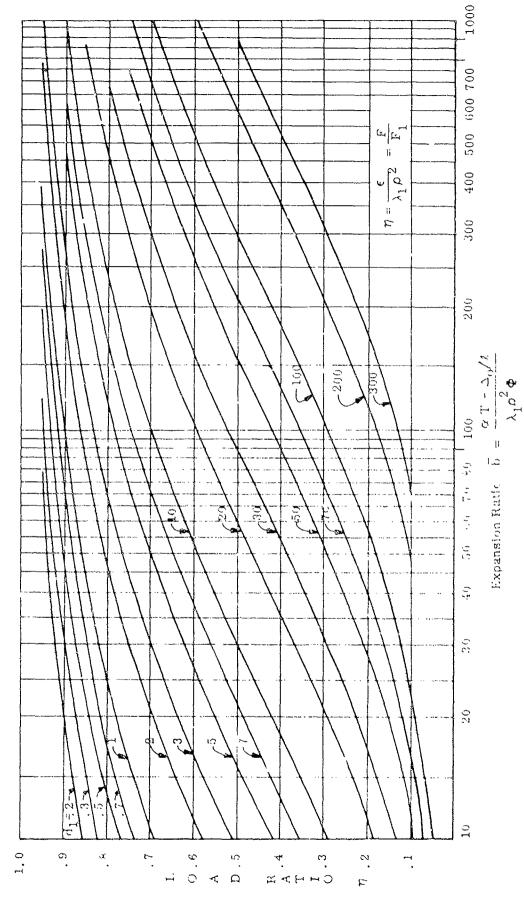


FIGURE 2, 2, 4-2 ANIAL LOAD IN A RUSTRAINED COLUMN

2. 2. 4 (Cont'd)

be employed to obtain the axial load acting on the column and the deformations of the column. It should be noted that the approximate method results in an upper bound on the axial load since it ignores a small part of the change of axial motion due to the higher modes.

2.3 PROCEDURE

The information and technique needed to solve the problem are enumerated in the following sections.

2.3.1 Determination of Nondimensional Parameters

By the definition Eq. (6a) of Paragraph 2.2.4:

$$d_{i} = \frac{\frac{1}{2} \int_{Q}^{\ell} (w_{io}^{\dagger})^{2} dx}{\ell \epsilon_{1}} = \frac{\Delta_{i}}{\ell \lambda_{i} \rho^{2}}$$

Equation (9, 1, 2 - 3e) of Reference 2-2 indicates that

$$\lambda_{i} = \frac{F_{i}}{EI} = \frac{\int_{0}^{L} \varphi(w_{i}^{"})^{2} dx}{\int_{0}^{L} (w_{i}^{"})^{2} dx} = \frac{\int_{0}^{L} \varphi C_{i}^{2} x_{i}^{2} dx}{2 \Delta_{i}}$$
(1a)

$$\therefore \Delta_{i} = \frac{C_{i}^{2} \int_{0}^{l} \varphi x_{i}^{2} dx}{2\lambda_{i}}$$
 (1b)

and

$$d_{i} = \frac{C_{i}^{2} \int_{0}^{L} C x_{i}^{2} dx}{2 \ell \lambda_{1} \lambda_{i} \rho^{2}}$$
(2a)

Noting that

$$\frac{\eta_1}{\eta_1} = \frac{\epsilon/\epsilon_1}{\epsilon''\epsilon_1} = \frac{\epsilon_1}{\epsilon_1} = \frac{\lambda_1 \,\rho^2}{\lambda_1 \,\rho^2} = \frac{\lambda_1}{\lambda_1} \tag{3}$$

$$\therefore 2\frac{\eta_i}{\eta_1} d_1 = \frac{c_1^2 \int_0^t c_2 \int_1^2 dx}{c_1^2 \int_0^2 c_2}$$
(4a)

2.3.1 (Cont'd)

d and $\frac{\eta_i^d}{\eta_1}$ will be evaluated for the chosen boundary conditions and for constant EI $(\varphi = 1)$ utilizing Eqs. (5) and (6) of Paragraph 2.2.3.

(1) Simple-Simple Column

$$d_{i} = \frac{C_{i}^{2}}{2\ell\left(\frac{\pi}{\ell}\right)^{2}\left(\frac{i\pi}{\ell}\right)^{2}\rho^{2}} = \frac{\ell^{4}}{4\pi^{4}\rho^{2}}\left(\frac{C_{i}^{2}}{i^{2}}\right)$$
(2b)

$$2 \frac{d_{i}\eta_{i}}{\eta_{1}} = \frac{C_{i}^{2} \ell/2}{\ell \left(\frac{i\eta}{\ell}\right)^{4} \rho^{2}} = \frac{\ell^{4}}{2\pi^{4}\rho^{2}} \left(\frac{C_{i}^{2}}{i^{4}}\right)$$
(4b)

(%) Clampe !- Clamped Column

Symmetrical

$$d_{1s} = \frac{C_{1s}^{2}}{2\iota \rho^{2} \left(\frac{2\pi}{\ell}\right)^{2} \left(\frac{2i\pi}{\ell}\right)^{2}} = \frac{\iota^{4}}{64\pi^{4} \rho^{2}} \left(\frac{C_{1s}^{2}}{i^{2}}\right)$$
(2e)

$$2\frac{d_{1s}\eta_{1s}}{\eta_{1}} = \frac{c_{1s}^{2} - t/2}{t\rho^{2} \left(\frac{21\eta}{\ell}\right)^{4}} = \frac{t^{4}}{32\eta^{4}\rho^{2}} \left(\frac{c_{1s}^{2}}{t^{4}}\right)$$
(4e)

Antisymmetrical

$$d_{ia} = \frac{C_{ia}^{2} \left(\frac{1}{2} - \frac{2 \cos u_{i}}{\mu_{i}}\right)}{2! \rho^{2} \left(\frac{2\pi}{l}\right)^{2} \left(\frac{2u_{i}}{l}\right)^{2}} = \frac{l^{4} C_{ia}^{2} \left(\frac{1}{2} - \frac{2 \cos u_{i}}{\mu_{i}}\right)}{32 \pi^{2} \rho^{2} u_{i}^{2}}$$
(2d)

$$2\frac{d_{ia} r_{ia}}{\eta_1} = \frac{c_{ia}^{-2} / \left(\frac{1}{2} - \frac{2\cos \mu_i}{\mu_i}\right)}{c_0^2 \left(\frac{2\mu_i}{T}\right)^4} = \frac{c_{ia}^{-2} \left(\frac{1}{2} - \frac{2\cos \mu_i}{\mu_i}\right)}{16 o^2 \mu_i^4}$$
(4d)

2.3.1 (Cont*d)

(3) Clamped-Simple Column

$$d_{i} = \frac{C_{i}^{2} \left(\frac{1}{2} - \frac{\cos u_{i}}{\mu_{i}}\right)}{2 \ln^{2} \left(\frac{1.43\pi}{\ell}\right)^{2} \left(\frac{\mu_{i}}{\ell}\right)^{2}} = \frac{\ell^{4} C_{i}^{2} \left(\frac{1}{2} - \frac{\cos 2\mu_{i}}{\mu_{i}}\right)}{4.1 \pi^{2} \rho^{2} \mu_{i}^{2}}$$
(2e)

$$2\frac{d_{i}\eta_{i}}{\eta_{i}} = \frac{C_{i}^{2} \ell\left(\frac{1}{2} - \frac{\cos 2\mu_{i}}{\mu_{i}}\right)}{\ell \rho^{2} \left(\frac{\mu_{i}}{\ell}\right)^{2}} = \frac{\ell^{4}}{\rho^{2}} \frac{C_{i}\left(\frac{1}{2} - \frac{\cos \mu_{i}}{\mu_{i}}\right)}{\mu_{i}^{4}}$$
(4e)

(4) Summary

Equation (9a) of Paragraph 2.2.4 can then be summarized for columns of constant EI as follows:

Simple-Simple

$$\frac{\alpha \Gamma - \Delta_0 / t}{\frac{\pi^2}{\ell^2} \rho^2} - \left(\frac{\ell^4 C_1^2}{4\pi^4 \rho^2} \right) \varphi_1 - \left(1 + \frac{EA}{k\ell} + \frac{\ell^4}{2\pi^4 \rho^2} \sum_{i=2}^{n} \frac{C_i^2}{i^2} \right) \eta_1 = 0 \quad (5a)$$

Clamped-Clamped Column

$$\frac{\alpha \Gamma - \Delta_0 / L}{\frac{4\pi^2}{\ell^2} \rho^2} - \left(\frac{\ell^4 C_1^2}{64\pi^4 \rho^2}\right) \varphi_1$$

$$-\left[1 + \frac{EA}{k!} + \frac{\frac{4}{32\pi^{4}\rho^{2}}}{32\pi^{4}\rho^{2}} \sum_{i=1}^{n} \frac{C_{i8}^{2}}{i^{4}} + \frac{\frac{4}{16\rho^{2}}}{16\rho^{2}} \sum_{i=1}^{n} \frac{C_{i}^{2}\left(\frac{1}{2} - \frac{\cos 2\mu_{i}}{\mu_{i}}\right)}{\mu_{i}^{4}}\right] = 0$$

(5b)

Simple-Clamped Column

$$\frac{\alpha T - \frac{\Delta_{0/I}}{2.05\pi^{2}\rho^{2}} - \left(\frac{.07081}{\pi^{4}\rho^{2}}\right)^{4} c_{1}^{-2}}{\pi^{4}\rho^{2}} \phi_{1} - \left[1 + \frac{EA}{k\ell} + \frac{\ell^{4}}{\rho^{4}} \frac{c_{1}^{2}}{2} - \frac{cos 2\mu_{1}}{\mu_{1}^{4}}\right] = 0$$
 (5e)

2.3.2 Computational Procedure

The following computational procedure is recommended to obtain the approximate solution for the axial load and lateral deflection of a restrained column.

(1) Determine the axial expansion of the column due to temperature and the lateral load, assuming, at this step, that the restraint against actual expansion is removed.

$$\ell \alpha T - \Delta_0 = \int_0^{\ell} \epsilon_T dx - \int_0^{\ell} (w_0^i)^2 dx$$
 (1)

where

$$w_0 = w_0 + w_T$$

 $w_q =$ lateral deflection due to lateral loads (usually obtained from reference texts)

 ^{W}T = lateral deflection due to thermal gradients (method of determining $_{T}$ indicated in Section 4 of Reference 2-2).

 $^{\epsilon}T$ = axial expansion due to temperature (method of determining ϵ_{T} indicated in Section 4 of Reference 2-2).

(2) Calculate the critical axial strain from Eq. (2c) of Paragraph 2.2.4)

$$\epsilon_1 = \lambda_1 \frac{1}{\Lambda} = \lambda_1 \, \varepsilon^2$$

(e.g.
$$\epsilon_1 = \frac{\pi^2}{\ell^2} \rho^2$$
 for simple-simple, $\varphi = 1$)

(3) Expand the initial curvature $(w_0^n + \sum C_i^- x_i^-)$, due to the lateral load and temperature, in terms of the characteristic curvatures of the column. A sufficient number of terms of the series should be taken to ensure accuracy. The coefficients of the characteristic curvatures are obtained by the general equation

$$C_{i} = \frac{\int_{0}^{1} \nabla w_{0}^{ir} x_{i} dx}{\int_{0}^{1} \nabla x_{i}^{2} dx}$$
 (2)

Formulas for constant EI (c -1) are presented in Eqs. (6a), (6b), (6c), and (6d) of Paragraph 2, 2, 3. Tables to evaluate the integrals of Eqs. (6a) and (6b) of Paragraph 2, 2, 3 are presented in Tables 2, 3, 3, 1, 2-1 and -2,

(4) Evaluate the pertinent parameters

(a)
$$d_i = \frac{\Delta_{io}}{\ell \epsilon_1} = \frac{C_i^2 \int c x_i^2 dx}{2 \rho^2 \lambda_1 \lambda_i^2}$$
 (Reference Eq. (2a) of Paragraph 2.3.1)

2.3.2 (Cont'd)

Appropriate formulas for d_i are presented for columns of constant EI and for the chosen boundary conditions in Paragraph 2.3.1.

(e.g.
$$d_i = \frac{\ell^4}{4\pi^4 \rho^2} = \frac{C_i^2}{\epsilon^2}$$
 for simple-simple column).

(b)
$$\Phi = 1 + \frac{EA}{k!} + 2 \sum_{i=2}^{n} \frac{\eta_i}{\eta_1} d_i$$
 (Reference Eq. (9b) of Paragraph 2.2.4)

Values of Φ are indicated in Eq. (5) of Paragraph 2.3.1 for constant EL

(e.g.
$$\Phi = 1 + \frac{EA}{k!} + \frac{l^4}{2\pi^4 \rho^2} \sum_{i=2}^{n} \frac{C_i^2}{i^4}$$
 for simple-simple column)

(c)
$$\overline{d}_1 = \frac{d_1}{\overline{\Phi}}$$
 (Reference Eq. (9f) of Paragraph 2.2.4)

(d)
$$\overline{b} = \frac{\alpha T - \Delta_{o/l}}{\epsilon_1 \overline{\Phi}} = \frac{\alpha T - \Delta_{o/l}}{\lambda_1 \rho^2 \Phi}$$
 (Reference Eq. (9e) of Paragraph 2.2.4)

- (5) Enter Figure 2.2.4-1 or -2 with the appropriate values of \vec{b} and \vec{d}_1 and determine η_1 .
 - (6) The axial load is then determined from Eq. (3a) of Paragraph 2.2.4.

$$F = \sigma A = E \frac{\epsilon}{\epsilon_1} \epsilon_1 A + E A \lambda_1 \rho^2 \eta_1$$
 (3)

(7) The lateral deflection of the column could likewise be determined as

$$\mathbf{w} = \sum_{i} \frac{\mathbf{w}_{0i}}{1 - \eta_{i}} \tag{4a}$$

where the value of
$$\eta_i = r_1 \frac{\lambda_1}{\lambda_i}$$
 (4b)

and the value of $\frac{1}{1-\eta_i}$ can be calculated or determined with the aid of Figure 2.2.1-1.

(e.g. w
$$-\frac{2}{\pi^2}\sum_{i=1}^{\infty} \left(\frac{t^2C_i}{2}\right) = \frac{\sin(i\pi x/t)}{i^2 - \eta_1}$$
 for pinned end column).

2.3.2 (Cont'd)

(8) A slightly better approximation to the value of n_1 can be obtained by employing

$$\overline{b} = \frac{\alpha T - \Delta_{o/\ell}}{\lambda_1 \rho^2} - \left(\frac{\Delta_o}{\ell \lambda_1 \rho^2} - \sum_{i=1}^n d_i\right)$$
(5)

which corrects for the error in approximating the initial shortening by a finite series. The additional shortening of the higher modes is still approximated by the first term of the series in order to simplify the solution of the compatibility equation.

(9) The graphs can also be employed to determine the approximate average temperature rise (with no cross-sectional gradient) required to produce a maximum permissible lateral deflection. The permissible ratio of maximum to initial lateral deflection $\left(\frac{w_{\text{max}}}{w_{\text{o}}}\right)$ can be employed with Figure 2, 2, 1-1 to estimate a value of η_1 which can be utilized with d_1 in Figures 2, 2, 4 to determine a value of \tilde{b} which could then be employed to calculate αT . (If the higher modes of the initial deflection are significant, then they must be included as indicated in Eq. (4a)). The initial eccentricity will permit the calculation of Φ and \tilde{d}_1 . Assuming a value of η_1 or of αT will permit the determination of the other from Figure 2, 2, 4. The value of η_1 could then be used to determine w which could be plotted against αT ; αT could also be plotted against the axial load by employing Eq. (3a) of Paragra 12, 2, 4. A typical plot is shown in Figure 2, 3, 2-1.

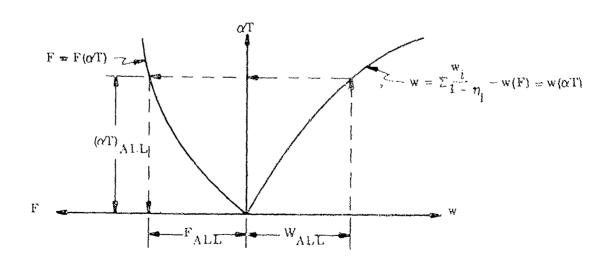


FIGURE 2, 3, 2-1 AXIAL LOAD AND LATERAL DEFLECTION VS AVERAGE TEMPERATURE RISE

2.3.3 Initial Deformations

The initial deformations before the end load is applied must be determined to a fair degree of accuracy. Two methods are presented herein. The first procedure assumes that an analytical expression for the initial lateral deflections is available as a polynomial, while the second method assumes that the lateral deflection is known only at a finite number of points (e.g., determined experimentally or by computations).

2.3.3.1. Polynomial Representation

The lateral deformation of a beam can be determined by integrating the equilibrium equation (Eq. (3) of Paragraph 2.2.2) with $\lambda = 0$ and employing the given boundary conditions. Solutions for lateral loading are available in literature for the type of boundary conditions con-

sidered (e.g.,
$$w_q = \frac{q!^4}{24 \, \text{EI}} \left[\left(\frac{x}{\ell} \right) - 2 \left(\frac{x}{\ell} \right)^3 + \left(\frac{x}{\ell} \right)^4 \right]$$
 for a simple-simple beam with uni-

form lateral load). The slopes and curvatures can be obtained by differentiating these polynomial expressions. The lateral deflection due to thermal gradients is not as readily available but can be derived quite simply by utilizing the equations of Section 4 of Reference 2-2. The technique is illustrated for the simple-simple and clamped-clamped beams.

2.3.3.1.1 Lateral Deformation of Beams by Temperature Gradients

Let the thermal gradient through the depth of the beam be expressed as a polynomial in the spanwise dimension.

e.g.
$$\frac{\alpha T_0 - \alpha T_1}{h} = -\frac{\Delta \alpha T}{h} = \frac{1}{\sqrt{2}} \sum m_j \left(\frac{x}{\ell}\right)^j = \frac{1}{\sqrt{2}} \sum m_j \xi^j$$
 (1)

where T_i and T_o are the temperatures on the positive and negative side of the column with a linear gradient (see Figure 2.1-1).

If the structure is statically determinate, as in a simple-simple beam, then the thermal gradient represents the total curvature (w^n_T) since no redundant forces are produced by the temperature. The slope and deflection can then be obtained by integrating this curvature and employing the given boundary conditions.

$$\therefore w = \frac{1}{\ell^2} \int_0^x \int_0^x \sum_j m_j \left(\frac{x}{\ell}\right)^j dx dx = \int_0^{\xi} \int_0^{\xi_0} \sum_j m_j \xi^j d\xi d\xi$$

$$w = \sum_j \frac{m_j \xi^{j+2}}{(j+1)(j+2)} + c_1 \xi + c_2$$
(2)

For simple supports w(0) = w(1) = 0

$$\therefore w = \sum_{i=1}^{n} \frac{m_i}{(i+1)(i+2)} - (\xi^{j+2} - \xi)$$
 (3a)

&
$$\mathbf{w}^{\dagger} = \frac{1}{\ell} \sum_{j} \frac{\mathbf{m}_{j}}{(j+1)(j+2)} \left[(j+2) \xi^{j+1} - 1 \right]$$
 (3b)

2.3.3.1.1 (Cont'd)

If the structure is statically indeterminate, as in the clamped-clamped beam, then the curvature is affected by the curvature caused by the redundant loads. Equations (4.2.2.5 - 7 and -8) of Reference 2-2 can be employed to determine the deformation of the clamped beams where subjected to a thermal gradient. The thermal gradient $\Delta \alpha T = \frac{\Delta \alpha T}{h}$

-m $_{j}\,\,\xi^{j}/\ell^{2}$ induces redundant loads P $_{o}$ and M $_{o}$ and the following total deformations result:

$$w_{T}^{n} = \frac{1}{2} \sum_{j} m_{j} \left[\xi^{j} - \frac{6j\xi}{(j+1)(j+2)} - \frac{2(1-j)}{(j+1)(j+2)} \right]$$
 (4a)

$$w_{T}^{*} = \frac{1}{r} \sum_{j=1}^{r} m_{j} \left[\frac{\xi^{j+1}}{j+1} - \frac{3j \xi^{2}}{(j+1)(j+2)} - \frac{2(1-j)\xi}{(j+1)(j+2)} \right]$$
(4b)

$$w_T = \sum_{j=1}^{m} \frac{m_j}{(j+1)(j+2)} \left[\xi^{j+2} - j \xi^3 - (1-j) \xi^2 \right]$$
 (4c)

2, 3, 3, 1, 2. Expansion in Characteristic Curvatures

The polynomial definition of curvature can be converted to a eigenvector expansion by means of Eqs. (2a) of Paragraph 2.2.2 and (2) of Paragraph 2.3.2.

$$w_{T}^{n} + w_{q}^{n} = w_{Q}^{n} = \sum C_{i} x_{i} \text{ (Refer to Eq. (2a) of Paragraph 2.2.2)}$$

$$C_{i} = \frac{\int_{0}^{f} \omega w_{Q}^{n} x_{i} dx}{\int_{0}^{f} \omega x_{i}^{2} dx} \text{ (Refer to Eq. (2) of Paragraph 2.3.2)}$$

For the simple-simple column with constant EI, this results in the simple Fourier sine expansion of the initial curvature.

$$C_1 = \frac{2}{t} \int_0^t w_0^{tt} \sin \frac{i\pi x}{t} dx = \frac{2}{t^2} \sum_{i=1}^{t} m_i \int_0^1 \xi^i \sin i\pi \xi d\xi$$
 (1a)

where

$$w_0^n = \frac{1}{\ell^2} \sum_j m_j \xi^j$$

The clamped-clamped column with constant EI and symmetrical loading results in a Fourier cosine expansion.

$$C_{1s} = \frac{2}{l} \int_{0}^{l} w_{0s}^{ij} \cos \frac{2i\pi x}{l} dx + \frac{2}{l^2} \sum_{i} m_{j} \int_{0}^{1} \xi^{j} \cos 2i\pi \xi d\xi$$
 (1b)

The curvature coefficients for the clamped-simple and the clamped-clamped columns with anti-symmetrical load must be similarly evaluated.

2.3.3.1.2 (Cont[†]d)

Tables 2.3.3.1.2-1 and -2 present the values of the integrals for the simple-simple and the symmetrical clamped-clamped columns.

TABLE 2.3.3.1.2-1 FOURIER EXPANSION OF MONOMIAL FOR SIMPLE-SIMPLE COLUMN

			S(j, i)			
7	1	2	3	4	5	6
0	, 6366	0	. 2122	0	. 12732	0
1	.3183	1591	.1061	07958	.06366	05305
2	. 1893	1591	. 1013	-,07958	.06262	05303
3	. 1248	1350	. 09894	07655	.06211	05215
4	.08814	1108	, 09241	07353	.06061	05125
5	. 06541	09078	. 08383	06988	.05862	05011
6	. 05038	07497	. 07489	06560	.05629	04872
7	. 03995	06258	.06647	06099	.05368	-, 04712
8	. 03243	05280	. 05889	05631	.05088	04537
9	. 02683	04503	. 05223	05176	.04799	-, 04350
10	.02256	03877	. 04644	04748	.04510	-, 04155

$$\mathbf{w}^{(i)} = \sum C_{i} \sin \frac{i\pi x}{t} = \frac{1}{t^{2}} \sum m_{j} \left(\frac{x}{t}\right)^{j} = \frac{1}{t^{2}} \sum m_{j} \xi^{j}$$

$$C_{i} = \frac{2}{t^{2}} \sum m_{j} \int_{0}^{1} \xi^{j} \sin i\pi \xi d\xi = \frac{2}{t^{2}} \sum_{j=1}^{n} m_{j} S(j, i) \qquad (2a)$$

$$S(j, i) = \int_{0}^{1} \xi^{j} \sin i\pi \xi d\xi$$

$$S(j, i) = \frac{\left[1 + (-1)^{j}\right] (-1)^{j+2} j!}{2(i\pi)^{j+1}} \qquad (1)^{i} \sum_{n=0, 2, 4} \frac{(-1)^{2}}{(i\pi)^{n+1} (j-n)!} \qquad (2b)$$

2.3.3.1.2 (Cont'd)

TABLE 2.3.3.1.2-2 FOURIER EXPANSION OF MONOMIAL FOR CLAMPED-CLAMPED COLUMN

			C (j, i)			
j	1	2	3	4	5	6
1	0	Ú	0	0	0	0
2	.05066	.01267	.00563	.00317	.00203	.00141
3	.07599	.01900	. 00844	. 00475	.00304	.00211
4	.08592	. 02437	.01107	. 00627	.00403	.00280
5	, 08815	. 02926	.01360	. 00777	.00500	.00349
6	.08669	. 03337	.01595	.00920	.00596	.00416
7	.08353	. 03655	.01809	. 01057	. 00688	.00482
8	. 07967	. 03883	.02000	.01185	.00777	. 00546
9	. 07563	.04033	.02166	.01304	.00862	. 00609
10	.07167	. 04120	.02308	.01414	. 00942	. 00669

$$\mathbf{w}^{ij} = \sum C_{ij} \cos \frac{2\pi i x}{\ell} = \frac{1}{\ell^2} \sum \mathbf{m}_{ij} \left(\frac{\mathbf{x}}{\ell} \right)^{ij} = \frac{1}{\ell^2} \sum \mathbf{m}_{ij} \xi^{ij}$$

$$C_{i} = \frac{2}{\ell^{2}} \sum_{j=1}^{\infty} \int_{0}^{1} \xi^{j} \cos 2 i \pi \xi d\xi = \frac{2}{\ell^{2}} \sum_{j=1}^{\infty} \sum_{i=1}^{\infty} m_{j} (C(j, i))$$
 (3a)

$$C(j, i) = \int_0^1 \xi^j \cos 2i\pi \xi \, d\xi$$

$$C(j, i) = \frac{\left[1 + (-1)^{j}\right](-1)^{j/2} j!}{2(2i\pi)^{j+1}} + \sum_{\substack{n=1,3,5}} \frac{j!}{(2i\pi)^{j+n}(j-n)!}$$
(3b)

2.3.3.2 Discrete Lateral Deformation

In some instances the lateral deflection is not known as a continuous function, but is determined analytically (see Paragraph 4.2.1 of Reference 2-2) or experimentally at a discrete number of points. The deformation of "n" discrete points can be employed to obtain an approximating expansion of "n" characteristic deflection shapes. The amplitude of the "n" characteristic deflections are determined by matching the displacements at the known points. The technique is illustrated for a simple-simple column with known deflections at the 1/6 points (n=5).

$$w(\xi_j) = w(\frac{x_j}{\ell}) = w_j = \sum_{i=1}^5 a_i \sin \frac{i\pi x}{\ell}$$
 (1a)

The symmetry of the odd characteristic deflections and the anti-symmetry of the even characteristics can be employed to reduce the order of the simultaneous equations necessary to solve for the amplitudes (a_i).

$$\frac{w_1 + w_5}{2} = a_1 \sin \frac{\pi}{6} + a_3 \sin \frac{3\pi}{6} + a_5 \sin \frac{5\pi}{6}$$

$$\frac{w_2 + w_4}{2} = a_1 \sin \frac{2\pi}{6} + a_3 \sin \frac{6\pi}{6} + a_5 \sin \frac{10\pi}{6}$$

$$w_3 = a_1 \sin \frac{3\pi}{6} + a_3 \sin \frac{9\pi}{6} + a_5 \sin \frac{15\pi}{6}$$

$$\frac{w_1 - w_5}{2} = a_2 \sin \frac{2\pi}{6} + a_4 \sin \frac{4\pi}{6}$$

$$\frac{w_2 - w_4}{2} + a_2 \sin \frac{4\pi}{6} + a_4 \sin \frac{8\pi}{6}$$
(1)

The solution is

$$\begin{pmatrix} \mathbf{a}_{1} \\ \mathbf{a}_{3} \\ = \\ \begin{pmatrix} .667 \\ \mathbf{a}_{5} \end{pmatrix} = \begin{pmatrix} .667 \\ 0 \\ 0 \\ .333 \end{pmatrix} \begin{pmatrix} \frac{\mathbf{w}_{1} + \mathbf{w}_{5}}{2} \\ \frac{\mathbf{w}_{2} + \mathbf{w}_{4}}{2} \\ \mathbf{w}_{3} \\ \end{pmatrix}$$

$$\mathbf{a}_{2} = \frac{1}{3,464} (\mathbf{w}_{1} + \mathbf{w}_{2} + \mathbf{w}_{4} + \mathbf{w}_{5})$$

$$\mathbf{a}_{4} = \frac{1}{3,464} (\mathbf{w}_{1} + \mathbf{w}_{2} + \mathbf{w}_{4} + \mathbf{w}_{5})$$

$$(1c)$$

2.3.3.2 (Cont'd)

It should be noted that
$$C_i = -\frac{i^2 \pi^2}{\ell^2} a_i$$
 (2)

Similar solutions can be obtained for different boundary conditions and for different types of known deformations.

A rapid solution can be obtained if one assumes that the lateral deflection is primarily in the fundamental mode (e.g., uniform thermal gradient on a simple-simple column). Thus the values of the nondimensional parameters can be simply determined with a minimum amount of computation. As an example, the simple-simple column equation can be expressed as

$$\frac{\alpha \Gamma - \frac{\Delta_0}{\ell}}{\frac{\pi^2 \rho^2}{\ell^2}} - \left(\frac{\frac{\Delta_0}{\ell}}{\frac{\pi^2 \rho^2}{\ell^2}} - \frac{w^2 (\ell/2)}{\frac{4 \rho^2}{\ell^2}}\right) - \frac{w^2 (\ell/2)}{4 \rho^2} \varphi_1 - \left(1 + \frac{EA}{k \ell}\right) \eta_1 = 0 \quad (3)$$

Since

$$\mathbf{w}(\ell/2) \sim \frac{C_1 \ell^2}{\pi^2}$$

Similar expressions can be derived for the other boundary conditions.

2, 3, 3, 3 Axial Shortening

The initial axial shortening due to the application of temperature and lateral load is determined as a function of the polynomial coefficients expressing the lateral deflection. The axial shortening is determined as a function of the actual lateral deflection rather than by a function of the amplitude of the characteristic modes, in order to improve the accuracy of the solution (see Eq. 5 of Paragraph 2, 3, 2).

Assume that the lateral deflection (w_0) is readily available and is expressible as a polynomial of the fourth degree or less. Higher degrees of polynomials can be treated in a manner similar to that indicated below. Differentiating the deflection results in the slope (w_0) which can be squared and integrated over the length of the column to obtain twice the axial shortening

i.e.
$$w_0 = a + b \xi + c \xi^2 + d\xi^3 + e \xi^4$$
 (1a)

$$w_0^{-1} = \frac{1}{\ell} \left[b + 2 c \xi + 3 d \xi^2 + 4 e \xi^3 \right]$$
 (1b)

$$2\Delta_0 = \left(\frac{1}{6}(w_0^{-1})^2 dx - \frac{1}{7} \left[b^2 + 2bc + \frac{6bd + 4c^2}{3} + 2bc + 3cd\right]\right)$$
 (1c)

$$\cdot \frac{16ce \cdot 9d^2}{5} + 4de + \frac{16e^2}{7}$$

2.3.3.3 (Cont'd)

Typical examples for simple-simple columns are presented.

For Constant Formal Gradient $\left(\frac{\Delta \alpha T}{h}\right)$

$$-\frac{\Delta\alpha\,T}{h} = \frac{1}{\ell^2} \,\,\mathrm{m_o}\,\,\xi^0$$

$$j = 0$$
 and $w_0 = \frac{m_0}{(0+1)(0+2)}$ $(\xi^{0+2} - \xi)$ from Eq. (3a) of Paragraph 2.3.3.1.1

$$w_{o} = \frac{m_{o}}{2} (\xi^{2} - \xi) = -\frac{t^{2} \Delta \alpha T}{2h} (\xi^{2} - \xi) = m_{T} (\xi - \xi^{2})$$

where

$$m_{T} = \frac{\ell^2 \Delta \alpha T}{2h}$$

Substituting $b = m_T$ and $e = -m_T$ in Eq. (1e)

results in

$$2\ell \Delta_{o} = \frac{1}{3} m_{T}^{2}$$
 (2a)

For Uniform Lateral Load (q)

$$w_{Q} = \frac{q \ell^{4}}{24 EI} \quad (\xi - 2 \xi^{3} + \xi^{4}) = m_{Q} (\xi - 2 \xi^{3} + \xi^{4})$$

$$m_{Q} = \frac{q \ell^{4}}{24 EI}$$

where

substituting b = $m_q^{}$, d = $-2 \, m_q^{}$, and e = $m_q^{}$

results in
$$2 (\Delta_o = .486 \text{ m}_q^2)$$
 (2b)

For Combined Uniform Load and Thermal Gradient

$$w_o = (m_T + m_q) \xi - m_T \xi^2 - 2 m_q \xi^2 + m_q \xi^4$$
 (2c)

results in

$$2(\Delta_{0}^{2}) = .333 \text{ m}_{T}^{-2} + .800 \text{ m}_{T}^{-1} \text{m}_{q}^{-2} + .486 \text{ m}_{q}^{-2}$$
 (2d)

2.3.3.3 (Cont[†]d)

It should be noted that any manufacturing or loading eccentricities will augment the lateral deflection parameters (C_i and d_i) by causing an axial shortening when the column is loaded, but should not be included directly in the Δ_0 (initial axial shortening because of thermal gradients and lateral loads) of the compatibility equation. The lateral eccentricities due to the various causes an additional and are included in the additional axial motion of the ends. The axial shortening however, is not linear but is proportional to the square of the deformations. Thus the initial axial shortening (Δ_0) must be computed as the difference of the axial shortening between the column including the manufactured eccentricities and the column containing only the manufactured eccentricities (w_{00}).

i.e.
$$\Delta_0 = \int_0^L (w_q^{-1} + w_T^{-1} + w_{00}^{-1})^2 dx = \int_0^L (w_{00}^{-1})^2 dx$$
 (3)

2.3.4 Spring Constant (k)

The value of k to be employed in the solution of the problem can significantly effect the magnitude of the resulting axial load. A column with zero axial restraint (k + 0) doe not develop any axial load. The load increases with increasing axial restraint until it reaches a maximum for complete restraint $(k = \infty)$.

The value of k is the value of the load produced by moving one end of the column a unit distance in the axial direction relative to the other end (assuming that the column offers no resistance). This is the stiffness coefficient defined in Section 4.2.5 of Reference 2-2. If both ends of the column are spring mounted, then the value of k is determined by putting the two end springs in series

i.e.
$$\frac{1}{k} = \frac{1}{k_1} + \frac{1}{k_2}$$

$$\therefore \qquad k = \frac{k_1 k_2}{k_1 + k_2}$$
(1)

where k - effective axial restraint

k₁,k₂ - axial restraint at the ends of the column

The method of determining the value of k in a composite structure is illustrated below for a truss joint.

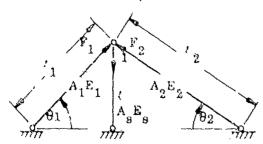


FIGURE 2.3,4-1 DETERMINATE TRUSS JOINT

The equilibrium equations are:

$$F_1 \sin \theta_1 + F_2 \sin \theta_2 = 1 \tag{2a}$$

$$F_1 \cos \theta_1 = F_2 \cos \theta_2 \tag{2b}$$

2.3.4 (Cont'd)

For symmetrical geometry $(\theta_1 = \theta_2 = \theta; A_1E_1 = A_2E_2 = AE)$

$$F_1 = F_2 = \frac{1}{2\sin\theta} \tag{2c}$$

$$\frac{1}{k} = \Delta = \frac{F \ell / \sin \theta}{AE \sin \theta} = \frac{\ell}{2AE \sin^3 \theta}$$
 (3a)

$$\frac{E_s A_s}{E_s} = \frac{1}{2 \sin^3 A} \left(\frac{E_s A_s}{EA} \right) \text{ where } E_s A_s = \text{axial stiffness of strut}$$
 (4a)

For unsymmetrical geometry

The load will cause a non-vertical motion of center strut and change the angles. For small deformations the following relationships result:

$$F_1 = \frac{1}{\sin \theta_1 (1 + \cot \theta_1 \tan \theta_2)}$$
; $F_2 = \frac{1}{\sin \theta_2 (1 + \cot \theta_2 \tan \theta_1)}$ (2d)

$$\frac{\ddot{\mathbf{I}}}{\mathbf{k}} = \Delta \sim \frac{1}{2} \left(\frac{\mathbf{F}_1 \ell_1}{\mathbf{A}_1 \mathbf{E}_1 \mathbf{sin} \theta_1} + \frac{\mathbf{F}_2 \ell_2}{\mathbf{A}_2 \mathbf{E}_2 \mathbf{sin} \theta_2} \right)$$

$$= \frac{\ell}{2} \left(\frac{1/A_1 E_1}{\sin^3 \theta_1 (1 + \cot \theta_1 \tan \theta_2)} + \frac{1/A_2 E_2}{\sin^3 \theta_2 (1 + \cot \theta_2 \tan \theta_1)} \right)$$
(3b)

Thus

$$\frac{E_{s}A_{s}}{kt} \sim \frac{E_{s}A_{s}}{2} \left(\frac{1}{A_{1}E_{1}\sin^{3}\theta_{1} \left(1 + \frac{\tan\theta_{2}}{\tan\theta_{1}}\right)} + \frac{1}{A_{2}E_{2}\sin^{3}\theta_{2} \left(1 + \frac{\tan\theta_{1}}{\tan\theta_{2}}\right)} \right)$$

This reduces to $\frac{E_g A_g}{kt} \sim \frac{1}{2 \sin^3 \theta} \left(\frac{E_g A_g}{EA} \right)$ for symmetrical geometry.

2.3.5 Effect of Non-Linearity in Spring or Material

Solutions to the axial load in the column can be obtained even when the axial restraint and/or stiffness of the column varies with the axial load.

The solution of a column with a variable axial restraint can be obtained by superimposing a plot of the flexibility parameter $\left[EA/\ell k = EA/\ell k \left(\eta_1 \right) \right]$ as a function of the axial load parameter $\left(\eta_1 \right)$ upon a plot of the solution of the compatibility equation for the load parameter $\left[\eta_1 = \eta_1 \left(\frac{EA}{\ell k} \right) \right]$ as a function of the flexibility parameter. The first plot is obtained directly from the spring characteristics whereas the second plot is obtained by varying the value of EA/k ℓ to obtain different values of η_1 . The intersection of the two plots, as illustrated in Figure 2.3.5-1, will result in a compatible solution.

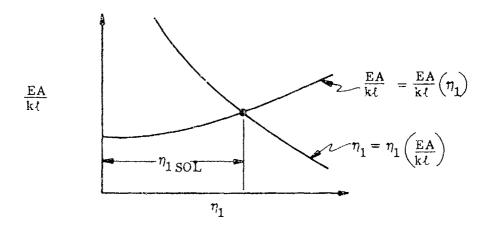


FIGURE 2.3.5-1 FLEXIBILITY VS AXIAL LOAD

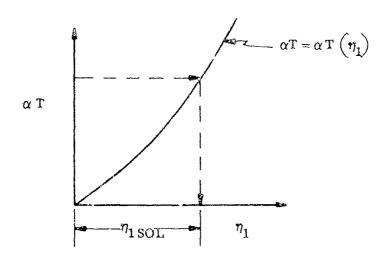


FIGURE 2.3.5-2 AXIAL LOAD VS AVERAGE THERMAL EXPANSION

2.3.5 (Cont[†]d)

the plasticity of the material can similarly be obtained by a trial and error procedure. Assuming a value of η_1 will determine a value of E_S . This will determine a value of the flexibility parameter $\frac{E_S}{k\ell}$ which varies linearly with E_S ; values of the lateral deflection parameters (C_i) whose lateral load component varies inversely with E_S and whose thermal component is unaffected; and the axial shortening parameters (d_i) which varies as the square of the C_i. Corresponding to these values of η_1 , values of \overline{d}_1 can be computed and employed in Figures 2.2.4 to obtain values of \overline{b} which can be utilized to calculate α T. The

The solution of a column with a load dependent material stiffness (Es) due to

This above method or be utilized to solve a problem with a load-dependent axial restraint and a load dependent material stiffness. The above technique assumes that the axial and bending stiffness of the column is not significantly affected by the variation of the stress through the thickness of the cross section but is determined primarily by the mean stress (F/A).

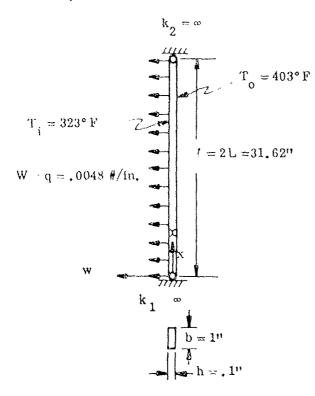
actual value of η_1 results when the calculated value of αT corresponds to the correct

value of αT . A graphical approach is illustrated in Figure 2.3.5-2.

2.3.6 Illustrative Problem

The computation procedure for the approximate solution is relatively simple and is illustrated for a problem which can be directly compared to an "exact" solution obtained from Table 1.5-1.

The temperature on each face is constant along the length with a linear gradient through the thickness.



$$k = \infty$$
 $\ell = 2L = 31.62 = 10 \sqrt{10}$
 $h = .1$
 $0 = 6(10)^{-6} \text{ in/in/oF}$
 $E = 12(10)^{6} \text{ pounds per square inch}$
 $\Delta T = T_{1} - T_{0} = -80^{\circ} \text{F}$
 $\Delta T = \frac{T_{0} + T_{1}}{2} = 363^{\circ} \text{F}$
 $\Delta T = \frac{0}{2} = 363^{\circ} \text{F}$
 $\Delta T = \frac{0}{2} = 363^{\circ} \text{F}$

2.3.6 (Cont'd)

AXIAL EXPANSION

From Eq. (1b) of Paragraph 2.2.4

$$\alpha T = \alpha \frac{T_0 + T_1}{2} = 6(10)^{-6} 323 = .0021780$$

AXIAL SHORTENING

From Eq. (2d) of Paragraph 2.3.3.3

$$\frac{\Delta_{0}}{\ell} = \frac{1}{\ell^{2}} \left[\frac{1}{6} m_{T}^{2} + .400 m_{T}^{m_{q}} + .243 m_{q}^{2} \right]$$

but

$$m_T = \frac{\ell^2 \Delta \alpha T}{2h} = \frac{1000 \ 6(10)^{-6} \ (-80)}{2(.1)} = -2.4$$
"

and

$$m_{q} = \frac{q \ell^{4}}{24 ET} = \frac{+.0048 (10)^{6}}{(24)12(10)^{6} \frac{1}{12} (.1)^{3}} = +.2^{11}$$

$$\frac{\Delta_0}{\ell} = \frac{1}{1000} \left[\frac{1}{6} (2.4)^2 + .4(2.4)(-.2) + .243(.2)^2 \right] = 7.777 (10)^{\frac{4}{5}}$$

From Eq. (2c) of Paragraph 2.2.4

$$\epsilon_1 = \lambda_1 \rho^2 = \lambda_1 \left(\frac{1}{A}\right) = \frac{\pi^2}{12} \frac{h^2}{12} = \frac{\pi^2}{1000} \frac{(.1)^2}{12} = 8.225 (10)^{-6}$$

$$\frac{\alpha T}{\epsilon_1} = 264.8 \quad \text{and} \quad \frac{\Delta}{\epsilon_1} = 94.56$$

EXPANSION OF CURVATURE

From Eq. (2c) of Paragraph 2.3.3.3

$$w_o = (m_T + m_q) \xi - m_T \xi^2 - 2m_q \xi^3 + m_q \xi^4$$
 (1a)

...
$$w_0' = \frac{1}{\ell} \left[m_T + m_q - 2m_T \xi - 6m_q \xi^2 + 4m_q \xi^3 \right]$$
 (1b)

...
$$w_0'' = \frac{1}{\ell^2} \left[-2 m_T - 12 m_q \xi + 12 m_q \xi^2 \right]$$
 (1c)

2.3.6 (Cont'd)

Direct substitution in the Fourier sine expansion or the use of Table 2.3.3.1.2-1 will enable the determination of the magnitude of the curvatures. Direct substitution is employed in this illustration to obtain general equations for the case of uniform load and thermal gradients. The Fourier coefficients defined by Eq. (2b) of Paragraph 2.3.3.1.2 are

$$S(0,i) = \frac{1 - (-1)^{i}}{i \pi}$$

$$S(1,i) = -\frac{(-1)^i}{i\pi}$$

and

$$S(2,i) = \frac{-(-1)^{i}}{i\pi} + \frac{2}{(i\pi)^{3}} \left[(-1)^{i} - 1 \right]$$

From Eq. (2a) of Paragraph 2.3.3.1.2

$$C_{i} = \frac{2}{\ell^{2}} \sum_{j} \sum_{i} m_{j} \qquad S(j,i)$$

$$C_1 = \frac{2}{\ell^2} \left[(-2 \text{ m}_T) \left(\frac{1 - (-1)^1}{1 \pi} \right) + (-12 \text{ m}_q) \left(\frac{-(-1)^1}{1 \pi} \right) \right]$$

$$+(12 \text{ m}_{q}) \left(\frac{-(-1)^{1}}{1 \pi} + \frac{2}{(1 \pi)^{3}} \left[(-1)^{1} - 1\right]\right)$$

$$C_i = \frac{2}{\iota^2} \left[(-2 m_T) \frac{(1-(-1)^i)}{i \pi} - \frac{24 m_q}{(i\pi)^3} (1-(-1)^i) \right]$$

$$C_{i} = -\frac{4}{\ell^{2}} (1 - (-1)^{i}) \left[\frac{m_{T}}{1\pi} + \frac{12 m_{q}}{(i\pi)^{3}} \right]$$

$$C_{1(odd)} = \frac{-\ell^2 C_{2k+1}}{2} = \frac{4\pi}{2k+1} m_T + \frac{(48\pi^3) m_q}{(2k+1)^3}$$
 (2a)

and for
$$C_{1(even)}$$
 $C_{2k} = 0$ (2b)

2, 3, 6 (Cont[†]d)

The values of C1 are then calculated. The same values would be obtained from Table

2.3.3.1.2-1
$$\frac{C_{i} \ell^{2}}{2} = \sum \sum m_{i} S(j, i)$$

$$= 4/\pi (m_{i}) + 48/\pi^{3} (m_{q})$$

$$= 1.27324(-2.4) + 1.548076(+.2)$$

$$= -3.05578 + .30962 = -2.746$$

$$-\frac{\ell^2 C_3}{2} = -\frac{1}{3} (3.05578) + \frac{1}{9} (.30962) = -1.007$$

$$-\frac{t^2C_5}{2} = -\frac{1}{5} (3.05578) + \frac{1}{25} (.30962) = -.609$$

$$-\frac{\ell^2 C_7}{2} = -\frac{1}{7} (3.05578) + \frac{1}{49} (.30962) = -.435$$

PERTINENT PARAMETERS

From Eqs. (2b) and (4b) of Paragraph 2, 3, 1

$$d_1 = \left(\frac{t^2 C_1}{2}\right)^2 - \frac{1}{\eta^4} \rho^2 i^2$$
 and $\frac{d_1 \eta_1}{\eta_1} = \frac{d_1}{i^2}$

$$\therefore d_{i} = \frac{(2.746)^{2}}{\pi^{4} \cdot (.1)^{2}} = 92.895$$

similarly

$$d_3 = 1.388$$
 and $\frac{d_3 \eta_3}{\eta_1} = d_3/3^2 = .154$

$$d_5 = .183$$
 $\frac{d_5 \eta_5}{\eta_1} = d_5/5^2 = .007$

$$d_7 = .047$$
 $\frac{d_7 \eta_7}{\eta_1} = d_7/7^2 = .001$

2, 3, 6 (Cont'd)

and
$$\frac{3}{2}$$
 $\frac{1-2}{\eta_1}\frac{\eta_1^3}{1} = \frac{d_3}{3}^2 + \frac{d_5}{5}^2 + \frac{d_7}{7}^2 = .162$

Employing Eq. (9a) of Paragraph 2.24 and including correction for finite sum approximation of the initial shortening (Eq. (5) of Paragraph 2.3.2) results in

$$0 = \frac{\alpha T}{\epsilon_1} - \frac{\Delta_0}{\ell \epsilon_1} - \left(\frac{\Delta_0}{\ell \epsilon_1} - \sum_{i=1}^{8} d_i\right) - d_1 \varphi_1 - \left(1 + \frac{EA}{k\ell} + 2\sum_{i=2}^{8} \frac{\eta_i d_i}{\eta_1}\right) \eta_1$$
 (3)

Substituting the calculated values results in

$$0 = (264.8 - 94.555) - (94.555 - 94.513) - 92.895 \varphi_1 - (1.234) \eta_1$$

$$0 = 170,203 - 92,895\varphi_{1} - (1,234)\eta_{1}$$

$$0 = 137.93 - 75.28\varphi_{1} - \eta_{1} = \overline{b} - \overline{d}_{1} \varphi_{1} - \eta_{1}$$

Entering Figure 2.2.4-2 with $\overline{b}=137.93$ and $\overline{d}_1=75.28$ r hults in the solution $\eta_1=.407$.

AXIAL LOAD

From Eq. (3) of Paragraph 2.3.2

$$F = AE \epsilon_1 \eta_1 = (1.1) (1) (12) (10)^6 8.225 (10)^{-6} .407 = 4.017$$

LATERAL DEFLECTION

From Eq. (4a) of Paragraph 2.3,2

$$w_{\text{max}} = w(\ell/2) = \sum \frac{w_{\Omega i}}{1 - \eta_i} = -\frac{\ell^2}{\pi^2} \sum \frac{C_i}{i^2 (1 - \eta_i)} = -\frac{2}{\pi^2} \sum \frac{\ell^2 C_i}{2i^2 (1 - \eta_i)}$$

$$w_{\text{max}} \sim -\frac{2}{\pi^2} \left[\frac{-2.74616}{1-.407} + \frac{-1.00712}{9(1-.407/9)} + \frac{-.60867}{25(1-.407/25)} + \frac{-.43523}{49(1-.407/49)} \right]$$

$$w_{\text{max}} \sim .97^{\text{m}}$$

The geometry and loads employed in this illustrative problem results in parameters which correspond to an exact solution found in Table 1.5-1.

These parameters are

$$\overline{T}_{D} = \alpha \left(\frac{L}{h}\right)^{2} (T_{o} - T_{1}) = 12.0$$

$$\overline{W} = \frac{12W}{Eb} \left(\frac{L}{h}\right)^{4} = 3.0$$

$$\overline{T} = \alpha \left(\frac{L}{h}\right)^{2} (T_{o} + T_{1}) = 108.9$$

A comparison of the exact and approximate solutions is presented below.

	Exact Solution	Approximate Solution
$\overline{\lambda}$	- 1.000	- 1.005
$\eta_{1}^{}$.405	.407
F	4.01#	4.02#
Wmax	. 92"	. 97"

Note that the approximate solution results in slightly larger values. The approximate method, however can be applied to more general types of loadings and boundary conditions.

It should be noted that the illustrated example shown in Figure 1.7-1 of Section 1 is solved by an interpolation of the "exact" tabular values. The approximate graphical procedure as well as the interpolation of tables procedure is subjected to relatively greater errors in the vicinity of low load ratios. This is because of the greater relative significance of the higher modes in the graphic approach and the large slope of the \overline{T} vs $\overline{\lambda}$ (as illustrated in Figure 1.5-2 of Secti. 1 of the tabular solution which permits large variations in $\overline{\lambda}$ for small variations in \overline{T} . Fortunately the design of the column is not determined by this condition of low axial load. The interpolation and graphical solutions are not subject to significant errors, however, when the load ratios are higher.

A comparison of the interpolation solution and of the approximate graphical solution of this report is presented as follows:

	Interpolated Sol.	Approx. Graphical Sol.
$\overline{\lambda}$	524	607
η_1	.111	. 155
F	27,4	37.2
Wmax	. 16	. 18

2.4 REFERENCES

- 2-1. Switzky, H., "Approximate Solution For an Axially Restrained Column Subjected to Elevated Temperatures and Lateral Loads," Republic Aviation Corporation Report No. ARD-679-4, September 1961.
- 2-2. Switzky, H., Forray, M., and Newman, M. "Thermo-Structural Analysis Manual"-Volume I, Republic Aviation Corporation Report No. RAC 679-1, September 1960, revised November 1961 (to be published as WADD TR 60-517, Vol. I).

SECTION 3

APPROXIMATE SOLUTION FOR THE BUCKLING OF ECCENTRIC COLUMNS

bу

H. Switzky

SECTION 3

APPROXIMATE SOLUTION FOR THE BUCKLING OF ECCENTRIC COLUMNS

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SECTION 3 - APPROXIMATE SOLUTION FOR THE BUCKLING OF ECCENTRIC COLUMNS

3.1 SUMMARY

An approximate solution $\left(\overline{F}_1 = \frac{K}{\iota^2} E_{SO} I_O \left[1 - \left(\sqrt{6} \left(\frac{a_{10}}{r}\right) \left(1 - \frac{E_{TO}}{E_{SO}}\right)\right)^{2/3}\right]\right)$ is obtained

for the buckling of an eccentric column which includes the effects of the stress level, the stress-strain relationship, and the initial eccentricity.

The analysis technique is presented in a nondimensional form to permit the analyst to consider various cross sections, boundary conditions, materials and degrees of eccentricity. The type of cross section is reflected in the I/A ratio which defines the square of the radius of gyration (r). The boundary conditions are contained in the value of K_{f} ?

which consider the end fixity and length of the column. The material characteristics are reflected in material parameters (E_A , σ_o , β) which represent the stress-strain relationship.

The nondimensional analysis graphs are presented for several values of the eccentricity

 $\left(\frac{a_{10}}{r}\right)$ which may occur because of initial waviness, eccentric loading, lateral loads, or thermal gradients. Analysis for intermediate values of the eccentricity can be conducted by interpolation,

Illustrative problems are presented to indicate the computation procedure and the effect of the initial eccentricity upon the stability of the structure.

3.1.1 Definition of Symbols

The following symbols are used throughout this section:

- Original amplitude (eccentricity) of fundamental mode ⁸10
- Depth of cross section of the column h
- 1, Length of column
- Coefficient expressing initial deformations of the column as a power series m,
- Lateral load acting on column q
- Radius of gyration of cross section ı.
- Lateral deflection of column W
- Axial coordinate of column \mathbf{x}
- Distance from reference axis
- \bar{z}^{\dagger} Distance from reference exis to bending axis
- Distance from reference axis to centroidal axis $\frac{1}{\sqrt{2}} = \int z' dA / \int dA$
- 7. Distance from bending axis
- Area of cross section
- A Area of cross section C(j, 1) Fourier expansion coefficient $(C(j, 1) = \int_{0}^{1} \xi^{j} \cos 2\pi \xi d\xi)$
- $E = E_A$
- Initial slope of the σ vs. ϵ_{σ} curve. Secant modulus of the σ vs. ϵ_{σ} curve. $\left(\mathbf{E}_{\mathbf{S}} = \sigma/\epsilon_{\sigma}\right)$ $^{\rm E}{}_{\rm S}$
 - Secant modulus at the bending axis E_{So}
 - Tangent modulus, slope of the σ vs ϵ_{σ} curve $(E_{\tau} = \frac{\partial \sigma}{\partial \epsilon_{\sigma}})$ $E^{\mathbf{L}}$
 - Tangent modulus at the bending axis ETO

3, 1, 1 (Cont'd)

- \overline{EA} Axial stiffness of cross section $(\overline{EA} = \int E_S dA)$
- \overline{EI} Bending stiffness of cross section ($\overline{EI} = \int E_S^2 dA$)
- Buckling stiffness of cross section $\left(\overline{\overline{E}I} = \overline{EI} + \chi \frac{\partial \overline{EI}}{\partial \chi} = \overline{F}_1 / \frac{K}{\chi^2}\right)$
- F Axial load on structure (compression positive)
- F, Buckling load of column with zero eccentricity or a linear material
- \bar{F}_1 Buckling load of column $\left(\bar{F}_1 = \frac{K}{t^2} E_{So} I_o \left[1 \left(\sqrt{6} \left(\frac{a_{10}}{r} \right) \left(1 \frac{E_{To}}{E_{So}} \right) \right)^{2/3} \right] \right)$
- F_E Euler Buckling load $(F_E = \frac{K}{\sqrt{2}} EI_0)$
- I Second moment of area (inertia) of cross section $\left(1_0 = \int (z^1 \overline{z}_0)^2 d\Lambda\right)$
- K Stability constant depending apon boundary conditions and the bending stiffness

distribution along the columns
$$\left(K + t^2 \frac{\delta x}{\delta w} + \frac{t^2 \int_0^t \phi(w_1'')^2 dx}{\int_0^t \phi(w_1'')^2 dx}\right)$$

- M Moment acting on cross sections (Positive M causes compression in positive fibers).
- Q_o Third moment of area of cross section $\left(Q_o = \int \left(z^t \bar{z}_o^{-t}\right)^3 dA\right)$
- S(j,1) Fourier expansion coefficient $\left(S(j,1)-\int_0^1 \xi^j \sin\pi\xi \,d\,\xi\right)$
- T Temperature rise above datum
- α Coefficient of linear thermal expansion
- α Parameter expressing variation of secant modulus in the cross section

$$\left(\alpha - \frac{\mathbf{x}}{\epsilon_{O}} \left(1 - \frac{\mathbf{E}_{TO} \mathbf{y}}{\mathbf{E}_{SO}} \right) \right)$$

- β Nondimensional parameter employed in mathematical definition of stressstrain relationship (Section 3 of Reference 3-1)
- y Nondimensional parameter expressing the average variation of the tangent

modulus in the cross section
$$\left(y - \frac{z}{co} E_{T} dz - z E_{To}\right)$$

- Δo T Difference in thermal expansion
- δ Operator denoting a small variation
- € Axial strain of cross section
- ϵ_{σ} Axial strain caused by stress (ϵ_{σ} ϵ α T)
- $|\epsilon_1^{-}|$. Average axial strain corresponding to an axial load F $_1^{-}$
- ϵ_{o} Axial strain at the bending axis
- $\widetilde{m{\epsilon}}_1$. Average axial strain corresponding to an axial load $\widetilde{m{\epsilon}}_1$
- η Shift of bending axis $(\eta z_0' \overline{z}^t)$
- x Curvature
- ξ Nondimensional axial coordinate $(\xi \times \lambda)$
- α Axial stress of cross section (compression positive)

3.1.1 (Cont'd)

- σ_o Axial stress at bending axis
- σ₀ Reference stress in nondimensional stress-strain relationship (Section 3 of Reference 3-1)
- Nondimensional parameter expressing the variation of the bending stiffness along the length of the column $(\phi(x) = \overline{E1}/E_0I_0)$
- $\Phi \qquad \text{Nondimensional parameter } \left(\Phi = \left(\frac{E}{A} / \sigma_o \right) \left(\frac{KI}{Ab^2} \right) = \frac{E_A \epsilon_1}{\sigma_o} \right)$
- $\Phi \qquad \text{Nondimensional parameter} \quad \overline{\Phi} = \left(\frac{E_{A}/\sigma_{o}}{Ab^{2}} \right) \left[1 \left\langle \sqrt{6} \left(\frac{a_{10}}{r} \right) \left(1 \frac{\overline{E}_{T}}{\overline{E}_{1}} \right) \right\rangle^{2/3} \right]$ $= \frac{E_{A}\overline{\epsilon}_{1}}{\sigma_{o}}$

SUBSCRIPTS

- M Caused by mechanical load
- T Caused by temperature
- 1 Pertaining to the first (fundamental) mode
- o Pertaining to initial or datum

3.1.2 Discussion of the Problem

The stability of a structure is a very complex problem. Exact solutions exist only for very few special cases. Approximations must be attempted when the rolution is complicated by variations (reductions) of the stiffness with the applied load because of the resulting non-linearity of the equation. The reduction in the stittness is caused by a non-linear structural material. The non-linear stress strain relationship reducts the stiffness of the structure by lowering the secant modulus and shifting the neutral axis.

Temperature, time, and the eccentricities of the structure tend to reduce the allowable magnitude of the applied load on the structure. Elevated temperatures reduce the stiffness of the structure, increase the non-linearity of the material because of plasticity at the stresses caused by the applied and thermal stresses, and usually increase the eccentricity of the structure. The axial forces acting through the eccentricity of the structure imposes moments upon the structure which increase at a greater rate than the applied load. These moments cause a variation of stresses through the cross sections which can reduce the bending stiffness. The larger the eccentricity the earlier the initiation of the non-linearity of the material and the lower the stability of the structure. The bending stiffness can also decrease with time because of creep of the material.

The approach, employed in this section, to external moment acting on the column (Figure 3, 1, 2-1) is to determine when the increase of external moment acting on the column cross sections tend to become greater than the increase of internal moment that can be generated by the cross sections. The axial load acting on the column at this time is the buckling load and is a function of the boundary conditions, the bending stiffness of the cross sections, and the rate that the bending stiffness is decreasing. The bending stiffness is a function of the cross-section, the magnitude of the stresses and their distribution and the stress-strain relationship. The bending stiffness usually decreases with an increase in the applied load, temperature, time, or eccentricity of the column. Thus the column becomes unstable when the applied load becomes equal to the buckling load which is determined by the bending stiffness distribution and its rate of decrease. This can occur by increasing the applied load and/or decreasing the magnitude of the buckling load. The buckling load is reduced by the accentricity, the temperature, and time as indicated previously.

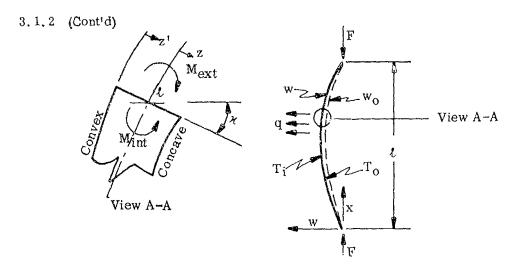


FIGURE 3.1.2-1 ECCENTRIC COLUMN SUBJECTED TO AXIAL LOALS

The evaluation of the buckling load requires the determination of the stiffness of the column and its rate of change. This is complicated by the shifting of the bending axis towards the convex side of the cross section because the higher stress and strain levels on the concave side reduce the secant modulus. The stiffness relationships, derived in Appendix A of Reference 3-2, are employed with the stability criterion described previously to obtain an approximate solution for the buckling load of the column in terms of the stress and lateral deflections. The solution is then approximated in terms of average stress level and initial eccentricity which can be readily computed. The initial eccentricities may result from the unloaded shape, eccentric loading, lateral loads, or thermal gradients. Methods of computing the initial eccentricity due to load and temperature are presented in Section 2 and summarized in this section.

An analogous approach can be employed to transform the loads or stresses to strains. Thus the column becomes unstable when the average axial strain becomes equal to a buckling strain. This approach is most convenient for a nondimensional presentation of the analysis.

The solution is predicated upon the following assumptions:

- (1) The bending strains are small compared to the axial strains in the column.
- (2) The temperature distribution can result in deformations and reductions in the moduli but do not cause significant thermal stresses.
- (3) There is a one-to-one relationship between the stress (σ) and the strain (ε_{σ}) for each fiber of the column. This assumes that there is no stress reversal throughout the loading history. It is also assumed that the stress-strain relationship can be approximated to a sufficient degree of a curacy by the relationship $\left[\frac{E_{\Lambda}\epsilon}{\sigma_{\sigma}} + (1-\beta)\frac{\sigma_{\sigma}}{\sigma_{\sigma}} + \beta \sinh\frac{\sigma}{\sigma_{\sigma}}\right]$ defined and described in Section 3 of Reference 3-1.

The approximate solution, which results from these assumptions, is of a form which does not violate known solutions of special cases and is indicative of the structural behavior in experiments.

3.2 ANALYSIS

A column subjected to loads and temperature (Figure 3.1.2-1) must satisfy equilibrium. Thus the change in applied moment (M_{ext}) must be equal to the change in the internal moment (M_{int}). The changes in external moment arise from changes in the axial load (F) and the lateral deflection (w). (The effect of elastic supports in introducing changes in the moment is reflected in the value of the stability constant K). The change in the internal moment is evidenced by a change in the curvature (x_M) and a possible change in the bending stiffness (\overline{EI}).

Referring to Figure 3, 1, 2-1, we note the equilibrium requirements.

$$M_{ext} + M_{int} = 0 (1a)$$

and

$$-\delta (M_{ext}) = \delta (M_{int})$$
 (1b)

evaluating the change in moments, we obtain

$$-\delta (M_{ext}) = -\delta (Fw) = -\left[F(\delta w) + w (\delta F) \right]$$
 (1e)

and

$$5(M_{int}) = 5(\overline{EI} \times_{M}) - [\overline{EI}(\delta \times_{M}) + \times_{M} (\delta \overline{EI})]$$
 (1d)

$$\therefore -\left[F(\delta w) + w(\delta F)\right] = \left[E\widetilde{I}(\delta x_{M}) + x_{M}(\delta E\widetilde{I})\right]$$
 (1e)

If the load is monotonically increasing, then the column will fail when the buckling load (\tilde{F}_1) is applied. No additional incremental load can be applied so that the buckling load is defined by setting δF equal to zero. The above buckling criterion is equivalent to finding the load on a structure for which the deflection is undefined (Figure 9.1-1b of Reference 3-1). It is also equivalent to determining the load on the structure at which the ratio of $\delta F/\delta w$ is zero, where δF is the incremental load necessary to cause an incremental deflection (δw) . The latter buckling criteria $(\delta F/\delta w = 0)$ can also be employed to determine the stability of a column which exhibits creep.

Setting $\delta F \ = \ 0$ and $F \ = \ F_{\frac{1}{4}}$ in equation (1e) results in

$$-\left[\tilde{F}_{1}\delta w + w(0)\right] + EIox_{M} + x_{M}\delta EI$$
 (2a)

$$. . . . \dot{F_1} = - \left[\begin{array}{ccc} \overline{EI} \frac{\delta x_M}{\delta w} + x_M \frac{\delta \overline{EI}}{\delta w} \end{array} \right] = - \frac{\delta x_M}{\delta w} \left(\overline{EI} + x_M \frac{\delta \overline{EI}}{\delta x_M} \right)$$

$$F_{1} = -\frac{\delta x}{\delta w} \frac{M}{E I}$$
 (2b)

Thus the buckling load is a function of the ratio of the change of curvature to the change of lateral deflection and of the bending stiffness modified by an expression indicating the rate of change of bending stiffness with curvature.

The first expression δx_M , δw is relatively simple to calculate if we assume that the deformation modes of the given structure do not change as the axial load is increased to the buckling load. Thus it is assumed that a pin ended column of constant bending stiff

3. 2 (Cont'd)

ness, which buckles elastically in a sine wave, will buckle in a sine wave even if it becomes plastic. This condition is satisfied as long as the ratio of the bending stiffness distribution $\left(\varphi(x) = \frac{\widetilde{El}(x)}{E_0I_0}\right)$ along the column does not change significantly with the axial load. The stiffness can change due to plasticity but it is assumed that the φ ratio does not change significantly. This is consistent with our assumption that the bending stresses are small compared to the axial stresses so that the notice is inductive equal (for constant cross-rection area). Thus the value of $\delta x_{M}/\delta w$ is assumed equal to the value of the "linear" column. This is the classical stability coefficient found in various textbooks which can be expressed as the curvature to deflection ratio as indicated above or by the general expression of Eq. (1a) of Paragraph 2.3.1.

ratio as indicated above or by the general expression of Eq. (1a) of Paragraph 2.3.1.
$$-\frac{\delta \mathbf{x}_{M}}{\delta \mathbf{w}} = \frac{\mathbf{F}_{1}}{\overline{\mathbf{E}}\overline{\mathbf{I}}} = \lambda_{1} = \frac{\int_{0}^{t} \varphi(\mathbf{w}_{1}^{"})^{2} d\mathbf{x}}{\int_{0}^{t} (\mathbf{w}_{1}^{"})^{2} d\mathbf{x}} = \frac{K}{t^{2}}$$
(3)

Values of K can be found in various texts (e.g. References 3-3, and 3-4).

As an example for a pin-ended column of constant EI (i.e. $\varphi = 1$)

$$-\frac{\delta x_{M}}{\delta w} = -\frac{\delta \left(\frac{\pi^{2}}{\ell^{2}} \sin \frac{\pi x}{\ell}\right)}{\delta \left(\sin \frac{\pi x}{\ell}\right)} = \frac{\pi^{2}}{\ell^{2}} = \frac{K}{\ell^{2}}$$
(4a)

or
$$\lambda_{1} = \frac{\int_{0}^{\ell} (1) \left(-\frac{\pi^{2}}{\ell^{2}} \sin \frac{\pi x}{\ell}\right)^{2} dx}{\int_{0}^{\ell} \left(\frac{\pi}{\ell} \cos \frac{\pi x}{\ell}\right)^{2} dx} = \frac{\pi^{2}}{\ell^{2}} = \frac{K}{\ell^{2}}$$
 (4b)

$$\therefore K = \pi^2 \tag{4c}$$

The second expression $\left(\overline{\overline{EI}} = \overline{EI} + x_{\overline{M}} \frac{\delta \overline{EI}}{\delta x_{\overline{M}}}\right)$ is evaluated by utilizing the defi-

nition of the bending stiffness derived in Appendix A of Reference 3-2 which approximates the effects of the bending stress.

From Eq. (A10b) of Reference 3-2

$$\widetilde{EI} \approx E_{So} I_{O} (1 - 2\alpha^{2} r^{2})$$
 (5a)

where

$$\alpha = \frac{x_{M}}{\epsilon_{O}} \left(1 - \frac{r_{O}}{\epsilon_{SO}} \gamma\right) \tag{5b}$$

3. 2 (Cont¹d)

and
$$\gamma = \int_0^z E_T dz/z E_T$$
. (5c)

Substituting Eq. (3), (5a), and (5b) in Eq. (2b) results in

$$\vec{F}_1 = \frac{K}{L^2} E_{So}^{I}_{o} (1 - 6\alpha^2 r^2)$$
 (6)

The solution for the buckling load is now expressed in terms of geometry and boundary conditions (ℓ, I_0, r, K) , the stress level at the neutral axis (E_{So}, E_{To}) and the stress distribution

$$\left(\alpha = \frac{\varkappa_{M}}{\epsilon_{o}} \left(1 - \frac{E_{To}}{E_{So}} \gamma\right).$$
 Unfortunately the stress distribution parameter α is not speci-

fically defined but varies in a complex way with the applied load. It varies directly with the curvature (x_M) which increases nonlinearly with the load, inversely with the axial strain (ϵ_0) which is defined by the stress-strain relationship (Figure 3.2-1) and directly with the

stress distribution factor $\left(1 - \frac{E_{To}}{E_{So}} \gamma\right)$ which increase with load and curvature. Some ad-

ditional approximations are required to reduce the solution to a simple form expressible in terms of the initial conditions. Each of the approximations employed introduces small over- or under-estimates of the buckling load. It is hoped that the combined effect of all the approximations will esult in a reasonably accurate solution that considers the effects of eccentricity.

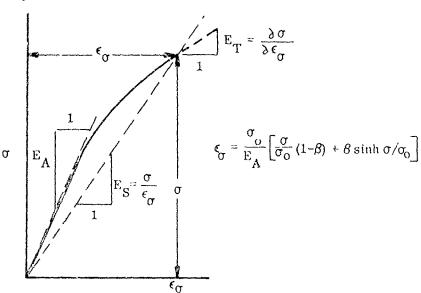


FIGURE 3. 2-1 STRESS-STRAIN RELATIONSHIP

The first approximation is to assume that the lateral deflection will increase as a rectangular hyperbola of the form

$$w = \frac{a_{10}w_1}{1 - F/F_1} \tag{7}$$

3. 2 (Cont'd)

where w = the lateral deflection

w₁ = fundamental deflection mode

a₁₀ = initial amplitude of the fundamental mode

(The higher modes need not be considered in the buckling load problem since the column will buckle in the fundamental mode)

F₁ = buckling load for a linear or straight column

where

$$F_1 = \frac{K}{\ell^2} E_{So}I_o \tag{8}$$

The second approximation is to assume that γ has the magnitude of unity in evaluating the value of α . Under the assumption of small bending stresses, the magnitude of γ should be very close to but slightly lower than 1.

The third approximation is to assume the average axial strain (ϵ_0) to be equal to the value of $F_1/E_{SO}A$ at buckling.

The above three approximations are consistent with the assumption of relatively small bending strains and result in a slight underestimation of α and a resulting slight overestimate of \overline{F}_1 .

The final approximation is to evaluate α with the stress distribution which exists at the cross section with the maximum lateral deflection. This assumption is made because the stability is primarily a function of the square of the curvature (see Eq. (3)) which is largest in the vicinity of the maximum lateral deflection (for columns of constant cross sections). This approximation of the bending to axial stress ratio overestimates α and results in an underestimate of \overline{F}_1 and compensates, to some degree, for the overestimates resulting from the small bending approximations.

The approximations result in the following equation

$$\bar{F}_{1} = \frac{K}{2^{2}} E_{So}^{I}_{o} \left[1 - \left[\sqrt{6} \left(\frac{a_{10}}{r} \right) \left(1 - \frac{E_{To}}{E_{So}} \right) \right]^{2/3} \right]$$
(9)

These approximations cannot be rigorously justified but they result in an expression for the stability of the column that can be simply applied to the analysis of a very complex problem and which retains the correct sense for the effect of the parameters. A well controlled test program is recommended to evaluate the accuracy of the resulting formulation and to supply empirical correction factors, as needed.

An examination of the experimental data of Reference 3-5 was conducted in Appendix C of Reference 3-2 and indicated good agreement with the results for specimens tested with eccentricities caused by eccentric loads or thermal gradients. The predicted results were slightly unconservative. Better agreement can possibly be obtained between the experimental results and the theoretical predictions by statistically determining the best value for the expression ($\sqrt{6}$ a₁₀/r). The graphs (Figures 3.3.1-1 through -10) can still be employed by modifying the indicated value of a₁₀/r. Additional test data is necessary, however, to examine the reliability of the above approximations for greater eccentricities and larger buckling stresses than those reported in Reference 3-5.

3.2 (Cont'd)

Using Eq. (9), the average buckling stress becomes

$$\overline{\sigma}_{1} = \frac{\overline{F}_{1}}{A} = E_{So} \frac{Kr^{2}}{t^{2}} \left[1 - \left[\sqrt{6} \left(\frac{a_{10}}{r} \right) \left(1 - \frac{E_{To}}{E_{So}} \right) \right]^{2/3} \right]$$
(10)

and the average buckling strain becomes

$$\overline{\epsilon}_{1} = \frac{\overline{\sigma}_{1}}{E_{So}} = \frac{Kr^{2}}{\ell^{2}} \left[1 - \left[\sqrt{6} \left(\frac{a_{10}}{r} \right) \left(1 - \frac{E_{To}}{E_{So}} \right) \right]^{2/3} \right]$$
(11a)

Since

$$\epsilon_{1} = \frac{\sigma_{1}}{E_{So}} = \frac{F_{1}/A}{E_{So}} = \frac{K}{\ell^{2}} \frac{E_{So}I_{o}}{E_{So}A} = K \frac{r^{2}}{\ell^{2}}$$
(11b)

$$\therefore \frac{\overline{\epsilon}_1}{\epsilon_1} = \left[1 - \left\{ \sqrt{6} \left(\frac{a_{10}}{r} \right) \left(1 - \frac{E_{To}}{E_{So}} \right) \right\}^{2/3} \right]$$
 (11c)

Letting

$$\overline{\Phi} = \frac{E_A}{\sigma_O} \overline{\epsilon_1}$$
 and $\Phi = \frac{E_A}{\sigma_O} \overline{\epsilon_1}$

We obtain from Eqs. (9), (10), and (11c)

$$\frac{\overline{F}_1}{F_1} - \frac{\overline{\sigma}_1}{\sigma_1} = \frac{\overline{\epsilon}_1}{\epsilon_1} = \frac{\overline{\Phi}}{\Phi} = \left[1 - \left(\sqrt{3} \left(\frac{a_{10}}{r}\right) \left(1 - \frac{E_{To}}{E_{So}}\right)\right)^{2/3}\right]$$
 (11d)

The factor
$$\left[1 - \sqrt{6} \left(\frac{a_{12}}{E_{S0}}\right) \left(1 - \frac{E_{To}}{E_{S0}}\right)^{2/3}\right]$$
 approximates the effect of the ec-

centricity and stress distribution. The effect is more pronounced with larger initial eccentricities or higher average stresses. It should be noted at this time that the factor was untermined for sections which were symmetrical about the centroidal axis parallel to the bending axis ($Q_0=0$). The stability of the column should decrease slightly with increasing values of O_0/I_0 as indicated by a comparison of the bending stiffness in the plastic range for $Q_0\neq 0$ with $Q_0=0$ (see Eqs. (A10a) and (A10b) of Reference 3-2 .

3.3 TECHNIQUE

The final formulation for the approximate solution for the buckling of an eccentric column is presented by the non-linear Eqs. (9), (10), or (11) of Sub-section 3.2. It is necessary to resort to graphical solutions in order to solve these equations. In order to simplify the analysis, a nondimensional solution is presented in Figure 3.3.1-1 to -10. The graphs are based upon the stress-strain relationship presented in Section 3 of Reference 3-1.

3.3.1 Nondimensional Buckling Curves

Equation (10) of Sub-section 3.2 can be transformed to

$$\left(\frac{\overline{\sigma}_{1}}{\sigma_{o}}\right) \frac{1}{\frac{E_{s}}{E_{A}} \left[1 - \left(\sqrt{6}\left(\frac{a_{10}}{r}\right)\left(1 - \frac{E_{To}}{E_{So}}\right)\right)^{2/3}\right]}$$

$$= \frac{E_{A}}{\sigma_{o}} \frac{Kr^{2}}{\ell^{2}} = \frac{E_{A}}{\sigma_{o}}\left(\frac{KI}{A\ell^{2}}\right) = \Phi$$
(1)

Utilizing the stress-strain relationship of Section 3 of Reference 3-1 $\left(\frac{E_A \epsilon}{\sigma_o} = (1-\beta) \frac{\sigma}{\sigma_o} + \beta \sinh^{\sigma}/\sigma_o\right)$, it is possible to calculate values of E_s/E_A and E_T/E_s for various values of σ/σ_o and given in values of β . For a given value of the eccentricity ratio (a_{10}/r) it is then possible to calculate the value of Φ , which would result in a buckling stress ratio $(\bar{\sigma}_1/\sigma_o)$, by direct substitution in Eq.(1). This relationship is plotted in Figures 3.3.1-1 to -10 for a large range of eccentricity ratios. The argument Φ is a function of the material properties (E_A/σ_o) and the geometry and boundary conditions $(KI/A\ell^2)$. The shape of the stress-strain relationship is represented by the value of β . The method of obtaining the material parameters from a uniaxial test is presented in Section 3 of Reference 3-1. The determination of the initial eccentricity which may be caused by initial waviness, eccentric loads, lateral loading, and thermal gradients is discussed in Paragraph 3.3.2.

The graphs for small eccentricity ratios should result in fairly good agreement with experimental data since the approximations employed should be satisfactory. The graphs for large eccentricity ratios are less accurate and are expected to be unconservative.

FIGURE 3, 3, 1-1 NON-DIMENSIONAL BUCKLING OF ECCENTRIC COLUMNS

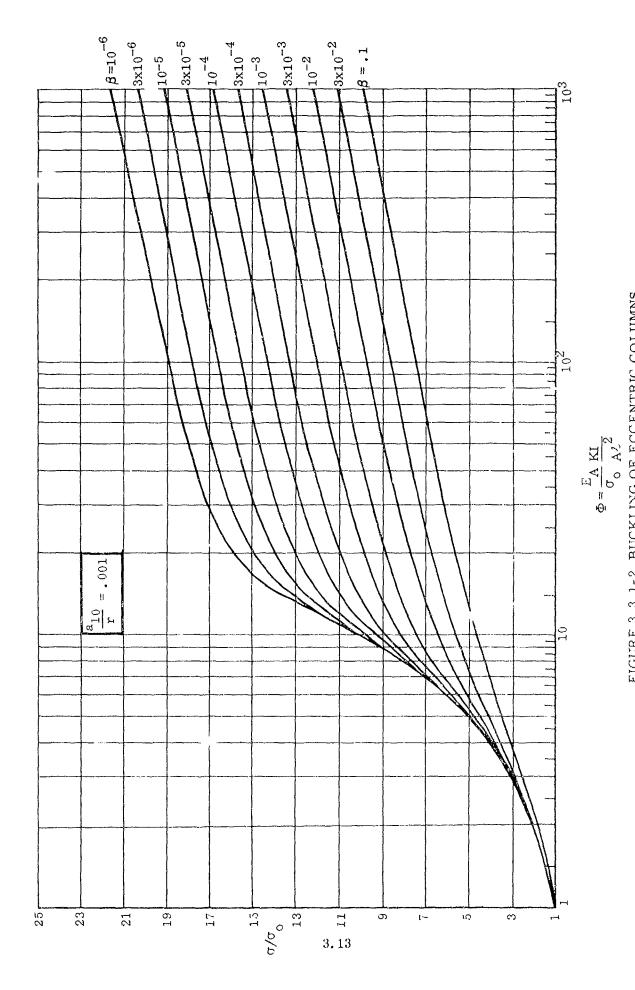


FIGURE 3. 3. 1-2 BUCKLING OF ECCENTRIC COLUMNS

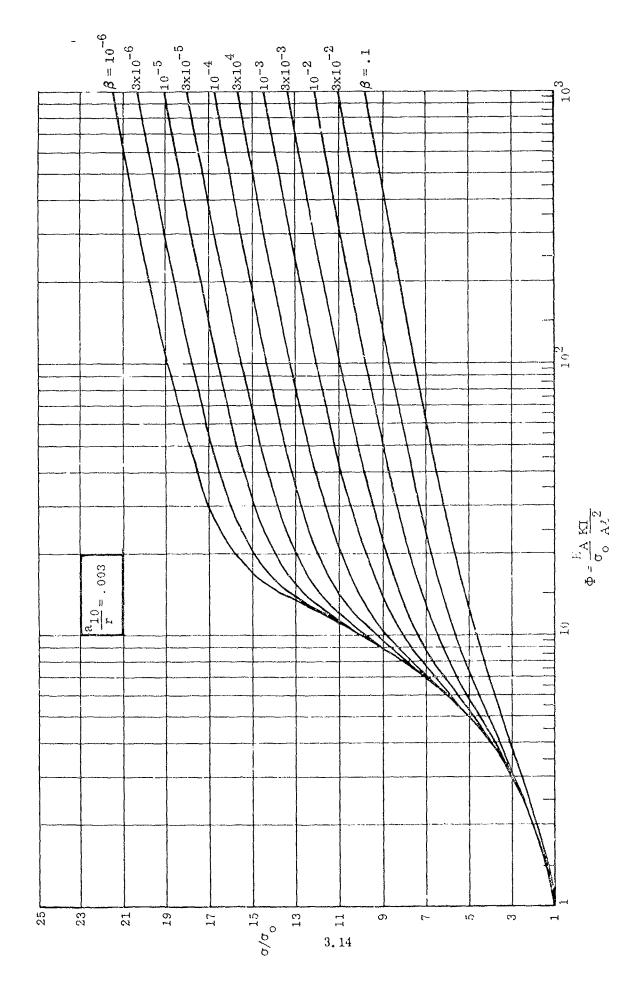
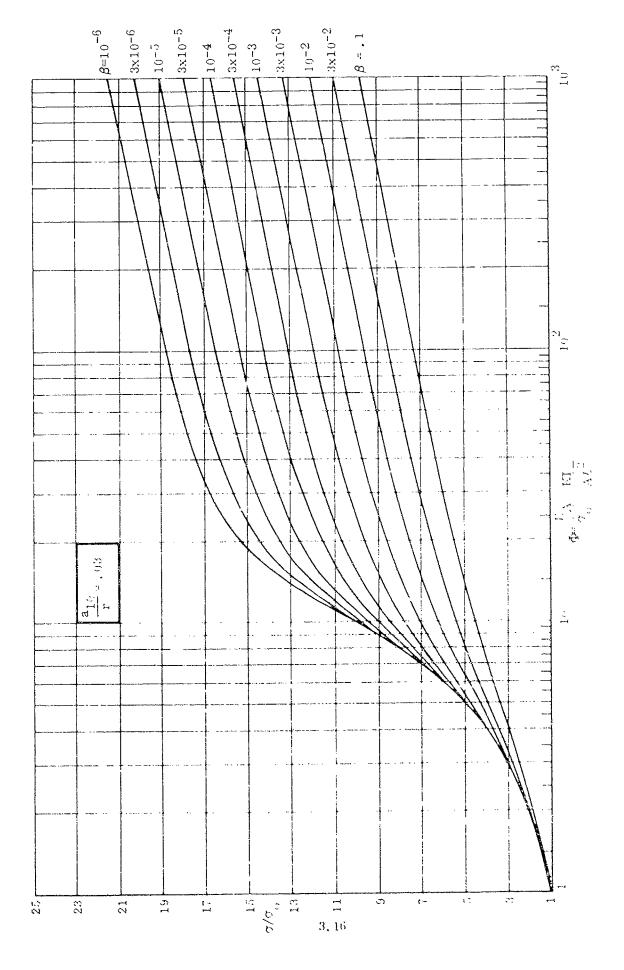
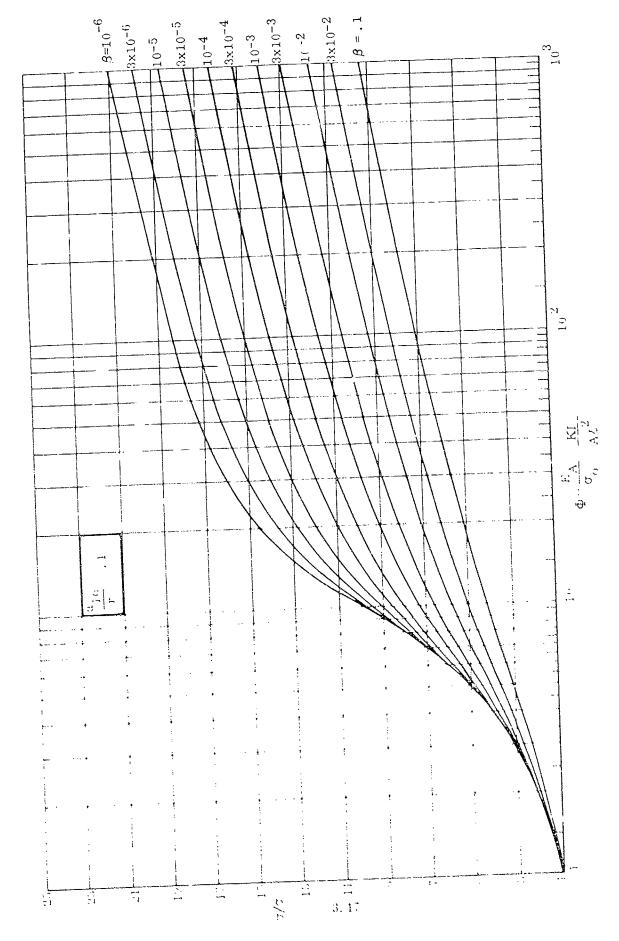


FIGURE 3, 3, 1-3 BUCKLING OF ECCENTRIC COLUMNS

FLURE 3.3.1-4 BUCKLING OF ECCENTRIC COLUMNS





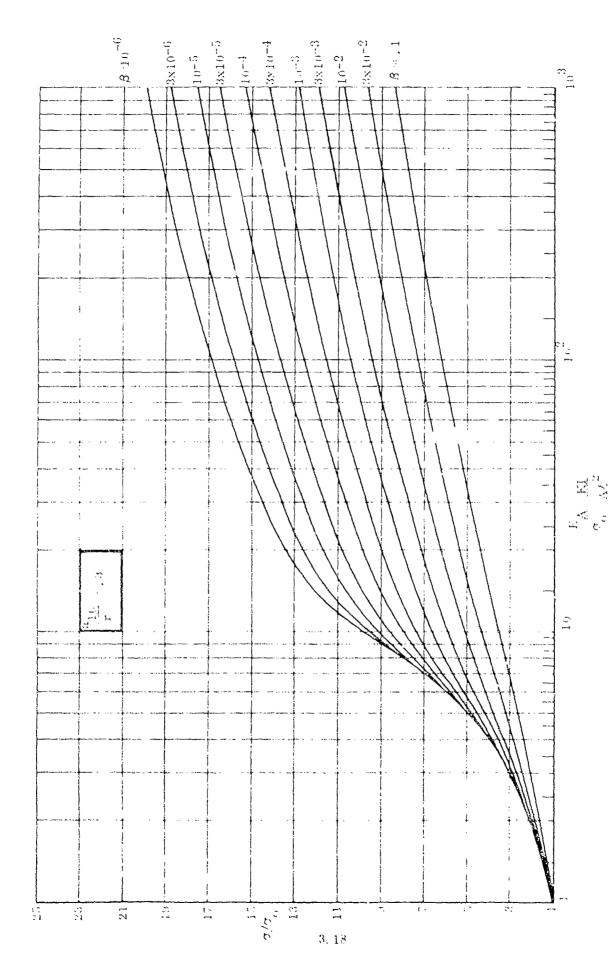


FIGURE 3, s. 1.7. BITCH ING OF ECCENTRIC COLUMNS

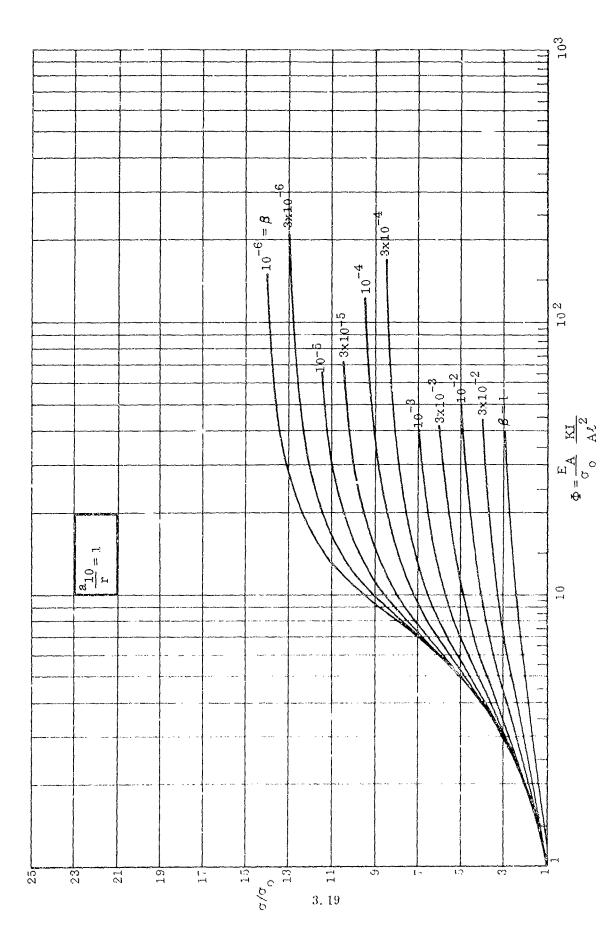


FIGURE 3.3.1-8 BUCKLING OF ECCENTRIC COLUMNS

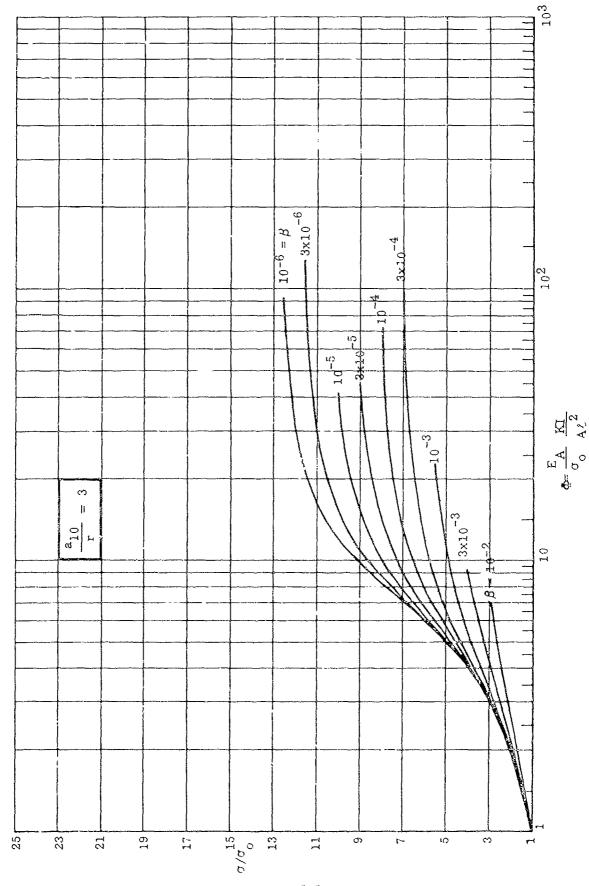


FIGURE 3.3.1-9 BUCKLING OF ECCENTRIC COLUMNS

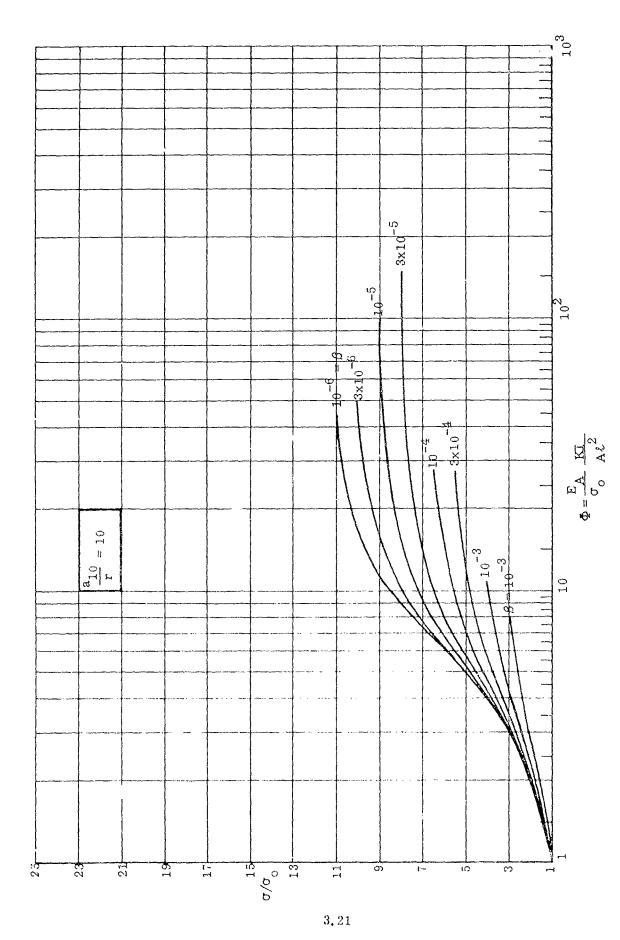


FIGURE 3. 3. 1-10 BUCKLING OF ECCENTRIC COLUMNS

3.3.2 Initial Eccentricity

The initial eccentricity must be evaluated in order to use the analysis curves.

Paragraph 2.3.3 indicated that the amplitude of the fundamental modes can be obtained by a weighted integration of the actual lateral deflection or by matching the deformation at discrete points.

Assuming that it is possible to express the initial eccentricity (wo due to shape, axial load location, lateral loads and temperatures) as an analytical expression, it is possible to determine the magnitude of the first mode by utilizing the orthogonality between the lateral deflection and curvature (Eq. (2) of Paragraph 2.2.1),

Thus, if

$$w_{o} = \sum a_{io} w_{i}$$
 (1a)

then from Eq. (2) of Paragraph 2.2.1 we obtain for constant bonding stiffness ($\omega=1$)

$$a_{io} = \frac{\int_{o}^{\ell} w_{o} \, \varkappa_{i} \, dx}{\int_{o}^{\ell} w_{i} \, \varkappa_{i} \, dx}$$
(1b)

The following formulae are applicable to columns of constant bending stiffnes;

when

$$w_{o} = \sum m_{j} \xi^{j}$$
 (2)

then for pin ends

$$a_{10} = 2 \sum m_i S(i, 1)$$
 (3a)

while for clamped ends

$$a_{10} = 2 \sum m_j C(j, 1)$$
 (3b)

when

$$x_{\mathrm{T}} = \frac{1}{\ell^2} \sum m_{j} \xi^{j} \tag{4}$$

3.3.2 (Cont'd)

then for pin ends (Reference Eq. (3a) of Paragraph 2.3.3.1.1)

$$w_{O} = \Sigma \frac{m_{j}}{(j+1)(j+2)} (\xi^{j+2} - \xi)$$
 (5a)

and

$$a_{10} = 2 \sum \frac{m_j}{(j+1)(j+2)} \left[S(j+2, 1) - S(1, 1) \right]$$
 (6a)

while for clamped ends (Reference Eq. (4c) of Paragraph 2.3.3.1.1)

$$w_{o} = \sum \frac{m_{j}}{(j+2)(j+2)} (\xi^{j+2} - j \xi^{3} - (1-j) \xi^{2})$$
 (5b)

and

$$a_{10} = 2 \sum_{j=0}^{m_j} \left[C(j+2, 1) - j C(3, 1) - (1-j) C(2, 1) \right]$$
 (6b)

Values of S(j,1) and C(j,1) are found in Tables 2.3.3.1.2-1 and -2 and are summarized in Table 3.3.2-1 below.

TABLE 3.3.2-1. FOURIER COEFFICIENTS FOR POLYNOMIALS

j	S(j, 1)	C(j, 1)
0	. 6366	-
1	.3183	0
2	.1893	. 05066
3	.1248	.07599
4	.08814	. 08592
5	.06541	. 08815
6	, 05038	. 08669

3, 3, 2 (Cont'd)

For a pinned end column of constant EI and constant linear thermal gradient

$$x_{\Gamma} = \frac{-\Delta \alpha T}{h} = \frac{m_0 \xi^0}{t^2} \text{ we obtain from Eqs.}(5a) \text{ and } (6a).$$

$$w_0 = \frac{m_0}{(0+1)(0+2)} \quad (\xi^{0+2} - \xi) = \frac{m_0}{2} \quad (\xi^2 - \xi)$$

$$\therefore w_0(\ell/2) = \frac{m_0}{2} \quad (.5^2 - .5) = -\frac{m_0}{8} = \frac{t^2 \Delta \alpha T}{8h}$$
and
$$a_{10} = 2 \frac{m_0}{(1)(2)} \left[S(2, 1) - S(1, 1) \right]$$

$$\therefore a_{10} = 2 \frac{m_0}{2} \left[.1893 - .3183 \right] = -.129 m_0 = \frac{.129 t^2 \Delta \alpha T}{h}$$
 (7)

A second method of obtaining the initial eccentricity is by matching the displacements at a discrete number of points. This requires the solution of the set of simultaneous equations $\mathbf{w}_{0}(\xi) = \sum \mathbf{a}_{10} \ \mathbf{w}_{1}(\xi)$ for the value of \mathbf{a}_{10} and is described in Paragraph 2.3.3.2. The approximation $\mathbf{a}_{10} \approx \mathbf{w}_{0}(1/2)$ is a solution where only one point, the deflection at the center of the column, was matched. The accuracy would increase with the number of displacements which are matched, although the matching of the center deflection of a pin ended column with a uniform lateral load or thermal gradient results in a satisfactory determination of the initial eccentricity. The accuracy also depends upon the form of the lateral deflection. For example, a matching of the mid-lengt s deflection for a pin-ended column results in a more accurate amplitude of the fundamental mode caused by a uniform thermal gradient (parabolic) than by a constant eccentricity.

3.4 SPECIAL CASES

There exist special situations for which the exact solution is known or for which engineering approximations have been accepted because of experimental data. The plausibility of the approximate formulation, presented in this section, is reviewed by degenerating it to those situations with which the analyst has had some experience.

3.4.1 Linear Material

A material whose modulus is independent of the stress levels $\left(\frac{\partial E_S}{\partial \sigma} = 0, E = E_T\right)$ = E_S is described as a linear material. The value of $(1 - E_T o/E_{SO})$ is identically zero

and no reduction of bending stiffness occurs. The eccentricity (a_{10}/r) does not affect the stability and buckling occurs when the average axial strain attains a value of $K\left(\frac{r^2}{\ell^2}\right)$ (with

the load equal to $F_E = \frac{K}{t^2}$ EI) provided no fiber is stressed to its ultimate strength.

This corresponds to the classical "Euler Column" whose buckling load and strain are well defined because of the linearity of the material beyond the buckling stress. A column with a large slenderness ratio (ℓ /r) would behave in the manner described. Although the nondimensional stress-strain relationship includes the linear case (letting $\beta=0$), it is recommended that the actual value of β for the structural material be used even for the case of low buckling stresses. This will permit an approximation of the effects of eccentricities in causing extreme fiber stresses that may be beyond the "proportional limit" even when the average stress is very low. The deviation of the stability of a structure composed of material which is non-linear to a slight degree from that of a linear material will be insignificant for small eccentricities but may become significant for large eccentricities.

3.4.2 Perfectly 'raight Columns

The perfectly straight column does not bend when subjected to an axial load. This condition is virtually impossible to attain experimentally but has some theoretical value as a mathematical model. The column will be stable as long as the axial strain remains

below the critical strain K $\frac{\mathbf{r}^2}{\ell^2}$ since \mathbf{a}_{10} is zero. Any lateral excitation of the column

below this value of the critical strain will dampen out whereas it will magnify and become excessively large when the critical strain is reached or exceeded by the column.

3.4.3 Small Initial Eccentricities

The effect of the eccentricity upon the stability of the column is small when the eccentricity ratio (a_{10}/r) is small. An upper bound upon the stability load will always be F_1 which corresponds to the employment of $E_{S_0}I_0$ as the effective buckling stiffness of the cross section and a critical strain of K r^2/ℓ^2 .

Under the conditions of continuous loadings (E $_{So} \ge E_{To}$) and small eccentricities (1 $\ge \left(\frac{\varkappa_{M} z}{\varepsilon_{o}}\right)^{2}$), the lower bound of the stability load will be $F_{1}\left(\frac{E_{To}}{E_{So}}\right) = \frac{K}{\ell^{2}} E_{To} I_{o}$ which

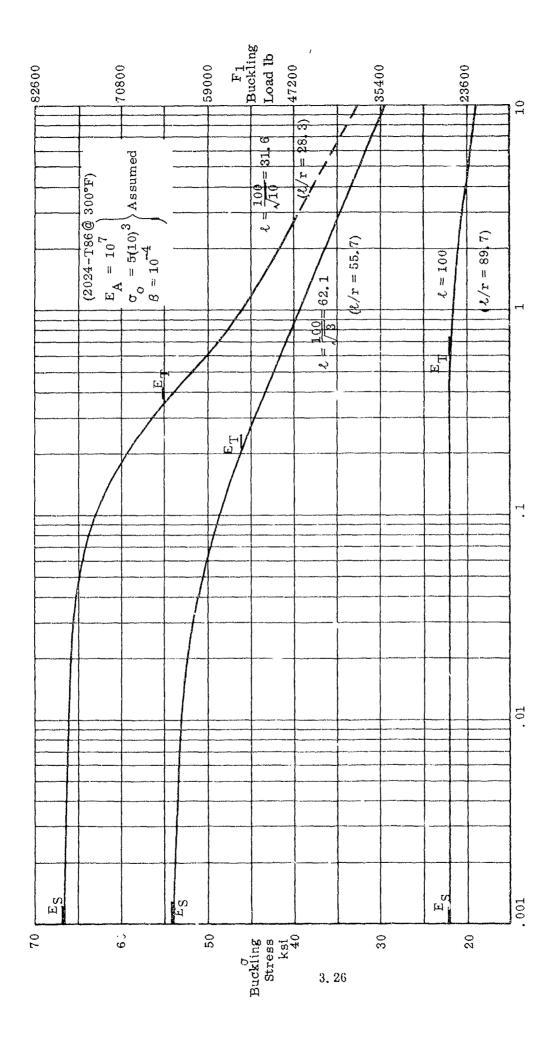


FIGURE 3. 4. 3-1 EFFECT OF ECCENTRICITY ON STABILITY

Eccentricity Ratio $\frac{a_{10}}{r}$

corresponds to the employment of $\mathbf{E}_{\mathbf{To}}^{}\mathbf{I}_{\mathbf{o}}^{}$ as the effective buckling stiffness of the cross

section and a critical strain of
$$\left(\frac{E_{To}}{E_{So}}\right) \left(K \frac{r^2}{\ell^2}\right)$$

This lower bound is satisfactory provided the eccentricity is not too large. The bending stresses become more significant in determining the bending stiffness as the eccentricity increases. An eccentricity ratio exists for each column beyond which the tangent modulus stability ceases to be a lower bound. This eccentricity ratio can be obtained by equating the tangent modulus load to the eccentric column load.

Equality results when
$$\frac{a_{10}}{r} = \sqrt{\frac{1 - E_{To}/E_{So}}{6}}$$

A value of (a_{10}/r) less than $\sqrt{\frac{1-\frac{E}{To}/E}{6}}$ results in a tangent modulus load

that is conservative with respect to the eccentric column load (i.e., $E_{To}I_0 \leq \overline{EI}$) whereas a larger eccentricity ratio will make the tangent modulus load unconservative. The value of the critical eccentricity ratio depends upon the material and the slenderness ratio of the column. This is illustrated in Figure 3.4.3-1 which indicates the effect of eccentricity on the stability of the columns described in the illustrative problems. The tangent modulus loads stops being conservative for eccentrivity ratios between .2 and .5 for the columns analyzed, and the critical eccentricity ratios are indicated in Figure 3.4.3-1.

3.5 ILLUSTRATIVE PROBLEMS

The computational techniques are illustrated in the following problems:

Find the buckling loads of the following pin ended columns of constant cross section illustrated in Figure 3.5-1.

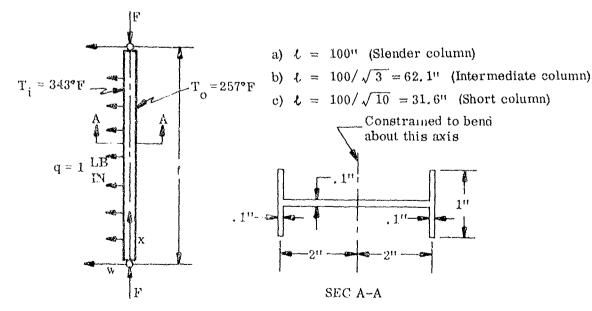


FIGURE 3.5-1 COLUMN SUBJECTED TO LOAD AND TEMPERATURE

The temperature is assumed constant on each face of the column with a linear gradient through the thickness. Material parameters are assumed for 2024-T86 at the mean temperature of 300°F to illustrate the computational technique. The actual values must be determined from standard material tests.

Material Properties

The following material properties are assumed:

$$\alpha = 12(10)^{-6} \text{ in/in/°F}$$

$$E_{A} = 10^{7} \text{ psi}$$

$$\sigma_{O} = 5000 \text{ psi}$$

$$\beta = 10^{-4}$$

Initial Geometry

A = 0.1 (1.0 + 3.9 + 1.0) = .59 in²

$$I_{o} = 2(.1)(2)^{2} + \frac{1}{12} (.1)(3.9)^{3} = 1.30 in^{4}$$

$$r^{2} = I_{o}/A = 2.2 in^{2}$$

$$r = 1.15 in$$

$$\Phi = \frac{E_{A}}{\sigma_{o}} \frac{KI}{A\ell^{2}} = \frac{E_{A}}{\sigma_{o}} K \frac{r^{2}}{\ell^{2}} = \frac{10^{7}}{5(10)^{3}} \pi^{2} \frac{2.2}{\ell^{2}} = \frac{4.4}{(\ell/100)^{2}}$$

$$\epsilon_{1} = K \frac{r^{2}}{\ell^{2}} = \pi^{2} \frac{2.2}{\ell^{2}} = \frac{22}{\ell^{2}}$$

Initial Eccentricity

Because the temperature distribution is linear through the thickness we can utilize Eq. (4) of Paragraph 3.3.2.

$$(a_{10})_{T} = .129 \frac{\Delta \alpha T}{h} \ell^{2} = .129 \frac{\alpha (T_{i} - T_{o})}{h} \ell^{2}$$

$$(1)$$

$$(a_{10})_{T} = \frac{.129 \alpha (T_{i} - T_{o}) \ell^{2}}{r h} = \frac{.129(12)(10)^{-6} (343 - 257)}{(1.15)(4.1)} = .28 \left(\frac{\ell}{100}\right)^{2}$$

From standard reference texts the lateral deflection due to a uniform load q is

$$(w_0)_q = \frac{q \ell^4}{24 \text{ EI}} (\xi - 2\xi^2 + \xi^4)$$

From Eq. (3a) of Paragraph 3.3.2

$$(a_{10})_q = 2 \frac{q!^4}{24 \text{ EI}} \left[S(1,1) - 2S(2,1) + S(4,1) \right]$$

From Table 3, 3, 2-1

It should be noted that

$$(w_0)_q$$
 (at $x = \ell/2$) = $\frac{5}{384} \frac{q \ell^4}{E I}$ = .01302 $\frac{q \ell^4}{E I} \approx$.013066 $\frac{q \ell^4}{E I}$ = $(a_{10})_q$

and

$$(w_0)_T$$
 (at $x = \ell/2$) = .125 $\frac{\Delta \alpha T}{h} \ell^2 \approx .129 \frac{\Delta \alpha T}{h} \ell^2 = (a_{10})_T$

This indicates the magnitude of the errors which can be introduced by approximating the amplitude of the fundamental mode by the deflection at the center of the column are quite small for a pin ended column subjected to uniform lateral load or thermal gradient.

The initial eccentricity ratio is then calculated as follows:

$$\left(\frac{a_{10}}{r}\right) = \frac{(a_{10})_{T}}{r} + \frac{(a_{10})_{q}}{r} = .28 \left(\frac{t}{100}\right)^{2} + .08 \left(\frac{t}{100}\right)^{4}$$

For

$$\ell = 100$$
 ; $\frac{e_{10}}{r} = .28 + .08 - .36$

$$\ell = 100/\sqrt{3}$$
; $\frac{a_{10}}{r} = .093 + .008 = .101$

$$\ell = 100/\sqrt{10}$$
; $\frac{a_{10}}{r} = .028 + .0008 = .0288$

Stability

Referring to the appropriate graph of Figures 3.3.1 results in the determination of $(\bar{\sigma}_1/\sigma_0)$ for the given values of β , Φ , and $(a_{10}/_r)$. The effect of the eccentricity, which is expressed in Eq. (11d) of Paragraph 3.2, can be evaluated by comparing the given value of Φ to the value $\bar{\Phi}$ which corresponds to $(\bar{\sigma}_1/\sigma_0)$ for a straight column (Figure 3.3.!-1

with $a_{10/r}=0$). This value of $\bar{\Phi}=\frac{E_A}{\sigma_0}$, corresponds to a material and geometry

parameter for a straight column which would buckle at the same stress as the eccentric column and is a measure of the average axial strain in the column.

a) For
$$\ell=100$$
, $E_A=10^7$, $\sigma_o=5000$, and $\beta=.0001$ we obtain $\Phi=4.4$, $\epsilon_1=.0022$, and $\frac{a_{10}}{r}=.36$.

From Figures 3.3.1-7 and -8

$$\frac{\ddot{\sigma}_1}{\sigma_0} = 4.4$$
 $\ddot{\sigma}_1 = 4.4(5000) = 22000 \text{ psi}$
 $\ddot{F}_1 = \sigma_1 \text{ A} = 22000 \text{ (.59)} = 13000 \text{ #}$

From Figure 3.3.1-1

$$\overline{\Phi} = 4.4$$

 $\Phi/\Phi = 4.4/4.4 = 1$ (no effect of eccentricity because of the large slenderness ratio)

and
$$\overline{\epsilon}_1 = \epsilon_1 (\overline{\Phi}/\Phi) = .0022$$

b) Similarly for $\ell = 100/\sqrt{3}$

$$\Phi = 13.2$$
, $\epsilon_1 = .0066$, and $\frac{a_{10}}{r} = .101$

From Flg... e 3.3.1-6

$$\frac{\overline{\sigma}_1}{\overline{\sigma}_0} = 9.7$$

$$\bar{\sigma}_1 = 9.7 (5000) = 48500 \text{ psi}$$

$$\overline{F}_1 = 48500(.59) = 28500 \#$$

and from Figure 3.1.1-1

$$\overline{\Phi} = 10.5$$
 $\overline{\Phi}/\overline{\Phi} = \frac{10.5}{13.2} = .80$

$$\therefore \overline{\epsilon}_1 = .0066(.80) = .0053$$
c) For $\ell = 100/\sqrt{10}$

$$\Phi = 44, \quad \epsilon_1 = .022, \text{ and } \frac{a_{10}}{a_{10}}$$

$$\Phi = 44$$
, $\epsilon_1 = .022$, and $\frac{a_{10}}{r} = .0288$
 $\therefore \overline{\sigma}_1/\sigma_0 = 13.1$
 $\overline{c}_1 = 10.1(5000) = 65500 \text{ psi}$
 $\overline{F}_1 = 65500(.59) = 38500 \text{ fs}$
 $\overline{\Phi} = 38 \quad \overline{\Phi}/\Phi = 38/44 = .87$
 $\overline{\epsilon}_1 = .022(.87) = .019$

The eccentricity ratios $a_{10/r}$ were sufficiently close to available values that there was no need to interpolate between graphs. Plots can be made to show the variation in the stability of a given column with the initial eccentricity and can be utilized if interpolation is required. These plots are shown in Figure 3.4.3-1 for the three (long, intermediate and short) columns analyzed above. The values corresponding to an effective modulus of E_8 are obtained from Figure 3.3.1-1 $\left(\frac{a_{10}}{r}=0\right)$ and the values corresponding to an effective modulus of E_T are obtained from Figure 9.2.1-2 of Reference 3-1. The plots indicate that $\overline{EI}=E_{80}$ is always unconservative and $\overline{EI}=E_{70}$ becomes unconservative in

the vicinity of $.2 > \frac{a_{10}}{r} \le .5$ for the analyzed columns. It should be noted that the reduction in the stability of the long column is least while the reduction in the stability of the intermediate column is greatest with the short column defected to an intermediate degree. This is because the reduction in the stability of the eccentric column depends both upon the eccentricity and the stress levels attained by the column. The long column has the largest eccentricity ratio but has very low axial stresses, the short column has very high axial stresses but very small eccentricities; while the intermediate column has high stresses and moderately high eccentricities.

It is interesting to note the contributions of the various factors in reducing the stability of the column below the "Euler Load" $\left(F_E - \frac{KEI_o}{\ell^2}\right)$. The factors reducing the stability stiffness can 1—roughly divided into two parts. The first part represents the reduction in the axial stiffness due to the plasticity caused by average stresses which are above the proportional limit of the materiai. The second part represents additional reductions in

the stability stiffness because of the eccentricity causing a shift in the neutral axis and a rate of change of the bending stiffness.

Equation (2b) of Paragraph 3.2 can be transformed to the following approximate form in order to evaluate quantitatively the destabilizing effects of eccentricity and plasticity.

$$\frac{\overline{F}_{1}}{F_{E}} = \frac{E_{So}{}^{I}{}_{o}}{EI_{o}} \left(\frac{\overline{EI}}{E_{So}{}^{I}{}_{o}} + \frac{x}{E_{So}{}^{I}{}_{o}} + \frac{x}{\partial \overline{EI}} \right) \approx \left(\frac{E_{So}{}^{I}{}_{o}}{EI_{o}} \right) \left(\frac{\overline{EI}}{E_{So}{}^{I}{}_{o}} \right) \left(1 - \frac{x}{E_{So}{}^{I}{}_{o}} \frac{\partial \overline{EI}}{\partial x} \right)$$

where

 \overline{F}_{1} is the buckling load of the eccentric column

F_E is the Euler buckling load

 $\frac{E_{So} I_{o}}{E I_{o}}$ is the factor representing the reduction in axial stiffness

 $\overline{EI/E}_{So}$ is the effect of the shifting of the neutral axis

 $\left(1 - \frac{x}{E_{So} I_{O}} \frac{\partial \overline{EI}}{\partial x}\right)$ is the effect due to the change in the bending stiffness

The numerical calculations for the destabilizing effects are summarized in Table 3.5-1.

TABLE 3.5-1 STABILITY RATIOS

DESTABILIZING PL. NOMENON	LENGTH OF COLUMN		
DEDITIONAL	ረ=100"	ℓ =100/√3	ℓ =100/√10
Reduction in Axial Stiffness			
(Plasticity Effect)			
$\frac{\frac{E_{So}I_{o}}{EI_{o}}}{\frac{F_{E}I_{o}}{EI_{o}}} = \frac{F_{1}}{F_{E}}$	1.00	.73	. 30
Further Reductions in Stability Stiffness			
(Eccentricity Effects)			
a) Shift of Neutral Axis			
$\frac{\overline{EI}}{E_{So}I_{o}} = 1 - 2\alpha^{2}r^{2}$	1,00	.93	. 955
b) Rate of Change of Bending Stiffness			
$1 - \frac{\chi}{E_{So}I_{o}} \frac{\delta E I}{\delta \chi} = 1 - 4 \alpha^{2} r^{2}$	1,00	. 87	.91
c) Critical Strain Ratio-Total Eccentricity Effect		:	
$\frac{\overline{\epsilon}_1}{\epsilon_1} = 1 - 6\alpha^2 r^2 = \frac{\overline{F}_1}{F_1}$	1.00	. 80	. 865
Stability Stiffness Reduction			
$\frac{\overline{F}_1}{\overline{F}_E} = \frac{\overline{F}_1}{\overline{F}_1} \frac{\overline{F}_1}{\overline{F}_{1.}}$	1.00	.58	. 26

3.6 REFERENCES

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SECTION 4

AXISYMMETRIC LARGE DEFLECTIONS OF CIRCULAR PLATES SUBJECTED TO THERMAL AND MECHANICAL LOADS

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SECTION 4 - AXISYMMETRIC LARGE DEFLECTIONS OF

CIRCULAR PLATES SUBJECTED TO THERMAL AND MECHANICAL LOADS

4.1 SUMMARY

This report is concerned with the nonlinear axisymmetric analysis of circular plates with in-plane edge restraint. Both temperature and mechanical loads are accommodated as an extension of investigations performed for the isothermal mechanical loading problem (References 4-1 through 4-4). An exact mathematical formulation within the framework of the v. Karman large strain-displacement relations (Reference 4-5) is developed. The equilibrium equations and boundary conditions are then derived by utilizing the calculus of variations for arbitrary axisymmetrical temperatures and normal distributed loading. The satisfaction of equilibrium and compatibility equations requires the solution of two simultaneous nonlinear ordinary differential equations subject to the prescribed boundary conditions. Analytical solutions of such equations are apparently not possible and therefore numerical procedures must be employed.

A finite difference procedure utilizing "relaxed iterations," developed by H. Keller and E. Reiss (Reference 4-4), and employed by them for the solution of isothermal problems with apparently unlimited load parameter ranges, is used here for combined thermo-mechanical problems. Numerical results are presented for the special case of a simply supported circular plate with radially immovable boundaries, subject to a uniform pressure and an arbitrary temperature variation through the thickness (no planform variation). These results have been obtained for a large range of temperature and load parameters. However, because of space limitations, only a limited amount of data is presented in this report.

4.2 INTRODUCTION

One of the basic assumptions of the classical linear theory of plates is that bending action does not induce significant midplane stretching. It is further assumed that stresses and deformations produced by loads and restraints in the midplane are superposible on the bending solution. Thus, coupling between the two effects is not accommodated by the classical theory. When the deflections are not small compared to the plate thickness, midplane stretching is no longer insignificant, resulting in a nonlinear interaction between bending and membrane stresses. Therefore, large deflection theory must be employed.

It is the purpose of this report to investigate the axisymmetric large deflection problem for circular plates.

The general formulation presented considers arbitrary axisymmetric temperature and pressure variation, where the von Karman large strain-displacement relations (Reference 4-5) are utilized. These assume infinitesimal strains and finite but small normal de-

flections ($(\frac{dw}{dr})^2$ is of the order of the strains, but small compared to unity). The remaining assumptions are those of classical plate theory. This formulation is more complete than the conventional linear theory in that the results are valid for deflection magnitudes several times the plate thickness. Moreover, buckling and postbuckling behavior are embodied in the analysis.

4.2 (Contid)

Numerical results in nondimensional tabular and graphical form are given for a simply supported circular plate, with full boundary restraint to radial movement, subjected to uniform pressure and arbitrary temperature variation through the thickness.

The following symbols are used throughout this section:

Ъ	Plate radius
h	Plate thickness
r	Radial coordinate
-	$\frac{\mathbf{r}}{\mathbf{b}}$, nondimensional radial coordinate
u	Midplane radial displacement
$\mathbf{u}_{\mathbf{b}}$	Midplane radial displacement at plate edge
u*	Radial displacement
W	Normal deflection
w	$\frac{\mathbf{w}}{\mathbf{h}}$, nondimensional normal deflection
Z	Thickness coordinate
D	$\frac{\mathrm{Eh}^3}{12(1-v^2)}$, flexural rigidity
E	Young's modulus
K	Relaxation parameter
$\mathbf{M_r}, \mathbf{M_t}$	Radial and tangential bending moments, respectively
$\overline{\mathtt{M}}_{\mathtt{r}}, \overline{\mathtt{M}}_{\mathtt{t}}$	Nondimensional radial and tangential bending moments, respectively
$\mathtt{M}_{\mathbf{T}}$	$h/2$ $\int E_{\alpha}T_{z}dz$ $-h/2$
$\overline{\mathtt{M}}_{\mathbf{T}}$	Nondimensional form of M _T
N _r , N _t	Radial and tangential membrane forces, respectively
$\overline{N}_{\mathbf{r}}, \overline{N}_{\mathbf{t}}$	Nondimensional radial and tangential membrane forces, respectively

 $egin{array}{ll} h/2 & & \int E lpha T dz \ -h/2 & & \end{array}$

 $\overline{\mathtt{N}}_{\mathbf{T}}$ Nondimensional form of $\mathtt{N}_{\mathbf{T}}$

q Normal pressure

Q Nondimensional normal pressure

T Local temperature with respect to an unstressed and undeflected datum

U_o Strain energy density

V Total potential energy

a Coefficient of linear thermal expansion

β Slope

 ϵ_r, ϵ_t Radial and tangential strains, respectively

 $\epsilon_{\mathbf{r}}^{0}, \epsilon_{\mathbf{t}}^{0}$ Midplane radial and tangential strains, respectively

λ Elastic in-plane edge restraint

v Poisson's ratio

 ϕ $\frac{\psi b}{D}$, nondimensional stress function

 ϕ_i Finite difference value of ϕ at the i'th grid point

Stress function

 σ_{n}, σ_{t} Radial and tangential stresses, respectively

 $\theta = \frac{b}{h} \sqrt{6(1-v^2)} \beta$

 θ_i Finite difference value of θ at the i'th grid point

4.3 BASIC EQUATIONS

4.3.1 Stress-Strain - Displacement Relations

The von Karman large strain - displacement relations for the axisymmetric strains at any point in the plate are given by

$$\epsilon_{\mathbf{r}} = \frac{d\mathbf{u}^*}{d\mathbf{r}} + \frac{1}{2} \left(\frac{d\mathbf{w}}{d\mathbf{r}}\right)^2$$

$$\epsilon_{\mathbf{t}} = \frac{\mathbf{u}^*}{\mathbf{r}}$$
(1)

where $u^*(r,z)$ is the radial displacement of the point and w(r) is the deflection normal to the undeflected midplane (Figure 4.3.1-1).

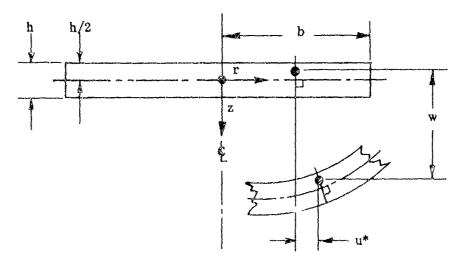


FIGURE 4.3.1-1 GENERAL AXISYMMETRIC DEFLECTION OF A TYPICAL POINT IN THE PLATE

Assuming plane sections to remain plane and normal to the deflected middle surface,

$$u^* = u - z \frac{dw}{dr}$$
 (2)

where u = u(r) is the radial displacement of the midplane. From (1) and (2),

$$\varepsilon_{\mathbf{r}} = \frac{d\mathbf{u}}{d\mathbf{r}} - \mathbf{z} \cdot \frac{d^{2}\mathbf{w}}{d\mathbf{r}^{2}} + \frac{1}{2} \left(\frac{d\mathbf{w}}{d\mathbf{r}}\right)^{2}$$

$$\varepsilon_{\mathbf{t}} = \frac{\mathbf{u}}{\mathbf{r}} - \frac{\mathbf{z}}{\mathbf{r}} \cdot \frac{d\mathbf{w}}{d\mathbf{r}}$$
(3)

The last (nonlinear) term in the first of (3) do_3 not appear in the conventional small deflection theory. From Hooke's law (neglecting normal stresses in the thickness direction), including the temperature "T", there results

$$\sigma_{\mathbf{r}} = \frac{E}{1-v^2} \left[\varepsilon_{\mathbf{r}} + v \varepsilon_{\mathbf{t}} - (1+v) \alpha T \right]$$

$$\sigma_{\mathbf{t}} = \frac{E}{1-v^2} \left[\varepsilon_{\mathbf{t}} + v \varepsilon_{\mathbf{r}} - (1+v) \alpha T \right]$$
(4)

An integration of (4) through the plate thickness yields

$$N_{\mathbf{r}} = \int_{-h/2}^{h/2} \sigma_{\mathbf{r}} dz = \frac{Eh}{1-v^2} \left[\varepsilon_{\mathbf{r}}^{O} + v \varepsilon_{\mathbf{t}}^{O} - \frac{(1+v)}{Eh} N_{\mathbf{T}} \right]$$

$$N_{\mathbf{t}} \int_{-h/2}^{h/2} \sigma_{\mathbf{t}} dz = \frac{Eh}{1-v^2} \left[\varepsilon_{\mathbf{t}}^{O} + v \varepsilon_{\mathbf{r}}^{O} - \frac{(1+v)}{Eh} N_{\mathbf{T}} \right]$$

$$(4a)$$

where the midplane strains are given by

$$\epsilon_{\mathbf{r}}^{O} = \frac{d\mathbf{u}}{d\mathbf{r}} + \frac{1}{2} \left(\frac{d\mathbf{w}}{d\mathbf{r}}\right)^{2} \\
\epsilon_{\mathbf{t}}^{O} = \frac{\mathbf{u}}{\mathbf{r}} ,$$
(4b)

In what follows, it is assumed that $\alpha T = \alpha T(r, z)$ but that E (and hence D) is constant.

and

$$N_{T} = \int \frac{E\alpha T dz}{-h/2}$$
 (4c)

Multiplying (4) by z and then integrating through the thickness yields

$$M_{\mathbf{r}} = \int_{-\mathbf{h}/2}^{\mathbf{h}/2} \sigma_{\mathbf{r}} z \, dz = -D \left[\frac{d^2 w}{d\mathbf{r}^2} + \frac{v}{\mathbf{r}} \cdot \frac{dw}{d\mathbf{r}} + \frac{\mathbf{M}_{\mathbf{T}}}{D(1-v)} \right]$$

$$M_{\mathbf{t}} = \int_{-\mathbf{h}/2}^{\mathbf{h}/2} \sigma_{\mathbf{t}} z \, dz = -D \left[\frac{1}{\mathbf{r}} \cdot \frac{dw}{d\mathbf{r}} + v \cdot \frac{d^2 w}{d\mathbf{r}^2} + \frac{\mathbf{M}_{\mathbf{T}}}{D(1-v)} \right]$$

$$(4d)$$

where

$$M_{T} = \int_{-h/2}^{h/2} E\alpha T z dz$$
 (4e)

and

$$D = \frac{Eh^3}{12(1-v^2)}$$

4.3.2 Strain and Potential Energies

The strain energy per unit of volume is given by (Reference 4-6):

$$U_{O} = \frac{1}{2} \left[\epsilon_{r} \sigma_{r} + \epsilon_{t} \sigma_{t} - \alpha T (\sigma_{r} + \sigma_{t}) \right]$$
 (1)

Substitution of the stress-strain relations (4) of Paragraph 4.3.1 into (1) yields

$$U_{o} = \frac{E}{2(1-v^{2})} \left[\varepsilon_{r}^{2} + \varepsilon_{t}^{2} + 2v \varepsilon_{r} \varepsilon_{t} - 2(1+v) (\varepsilon_{r}^{+} \varepsilon_{t}) \alpha T + 2(1+v) (\alpha T)^{2} \right]$$
 (2)

For rotationally workless restraints at the outer boundary, the total potential of the plate and external loading system is given by the following equation:

$$V = 2\pi \int_{0}^{b} r \int_{0}^{b} U_{o} dz dr - 2\pi \int_{0}^{b} qw r dr + \pi b \lambda u_{b}^{2}$$
(3)

where q is the normal pressure, λ is an elastic restraint per unit circumferential length to radial displacement of the boundary, and $u_b = u_{r=b}$. From Eqs. (3) of Paragraph 4.3.1, and (2) and (3) the potential energy in terms of the displacement components and temperature becomes

where
$$V = \frac{\pi E}{1 - v^2} \int_0^b r \left\{ \left[\frac{du}{dr} + \frac{1}{2} \left(\frac{dw}{dr} \right)^2 \right]^2 + \left(\frac{u}{r} \right)^2 + \frac{2v u}{r} \left[\frac{dv}{dr} + \frac{1}{2} \left(\frac{dw}{dr} \right)^2 \right] \right\} h$$

$$+ \left\{ \left(\frac{d^2 w}{dr^2} \right)^2 + \left(\frac{1}{r} \frac{dw}{dr} \right)^2 + \frac{2v}{r} \frac{dw}{dr} \frac{d^2 w}{dr^2} \right\} \frac{h^3}{12}$$

$$+ 2(1 + v) \left\{ \int_{-h/2}^{h/2} (\alpha T)^2 dz + \frac{M_T}{E} \left[\frac{d^2 w}{dr^2} + \frac{1}{r} \frac{dw}{dr} \right] \right\}$$

$$- \frac{N_T}{E} \left[\frac{du}{dr} + \frac{1}{2} \left(\frac{dw}{dr} \right)^2 + \frac{u}{r} \right] \right\} dr - 2\pi \int_0^b q w r dr + \pi b \lambda \left(u_b \right)^2$$

$$(4)$$

4.3.3 Governing Differential Equations and Boundary Conditions

The equilibrium equations and "natural" boundary conditions are now obtained by making the potential energy stationary with respect to variations of the displacements w and u; i.e. $\delta_{\mathbf{w}}V=\delta_{\mathbf{u}}V=0$. The first of the variations ($\delta_{\mathbf{w}}V=0$) yields the equilibrium equation

$$D\nabla^{4}w - \frac{1}{r}\frac{d}{dr}\left(rN_{r}\frac{dw}{dr}\right) = q - \frac{\nabla^{2}M_{T}}{1-\nu}$$
(1)

and the following boundary conditions:

$$(1) r = 0$$

Assumed regularity of the solution at the center requires that M_r and M_t be finite. This implies that $\frac{dw}{dr} = 0$ and $M_r = M_t$.

(2)
$$\frac{r=b}{}$$
 (i) w prescribed or $-\frac{d}{dr}\left[D\nabla^2 w + \frac{M_T}{1-\nu}\right] + N_r \frac{dw}{dr} = 0$

(ii) w' prescribed or
$$D\left[\frac{d^2w}{dr^2} + \frac{v}{r} \frac{dw}{dr}\right] + \frac{M_T}{1-v} = 0$$
.

The second of the variations $(\delta_{_{11}}V=0)$ results in

$$N_t - \frac{d}{dr} (r N_r) = 0$$
 (3)

with boundary conditions:

(1)
$$r = 0$$

$$N_r \text{ and } N_t \text{ finite. This implies that } u = 0 \text{ (hence } N_r = N_t). \tag{4a}$$

(2)
$$\frac{r=b}{\left(u+\frac{N_r}{\lambda}\right)}=0$$
 or u prescribed. (4b)

The two equations (1) and (3) contain three unknown functions of r; i.e., w, N_r and N_t . A third equation is obtained from the necessary condition that these three quantities yield a set of single-valued displacements, u and w. A statement of this requirement is obtained by eliminating u from Equation (4b) of Paragraph 4.3.1, which results in

$$\frac{\mathrm{d}\epsilon_{t}^{o}}{\mathrm{d}r} + \frac{\epsilon_{t}^{o} - \epsilon_{r}^{o}}{r} = -\frac{1}{2r} \left(\frac{\mathrm{d}w}{\mathrm{d}r}\right)^{2}. \tag{5a}$$

Substituting (4a) of Paragraph 4.3.1 into (5a),

$$\frac{\mathrm{d}}{\mathrm{d}\mathbf{r}}\left(N_{t} - \nu N_{r} + N_{T}\right) + \frac{(1+\nu)}{r}\left(N_{t} - N_{r}\right) + \frac{\mathrm{Eh}}{2r}\left(\frac{\mathrm{d}\mathbf{w}}{\mathrm{d}\mathbf{r}}\right)^{2} = 0 \tag{5b}$$

Equation (3) is automatically satisfied by a stress function ψ defined through the relations

$$\dot{q} = r N_{r}$$

$$\frac{dq}{dr} = N_{t} . \tag{6}$$

Substitution of (6) into Eqs. (1) and (5b) results in the following coupled set of nunlinear differential equations

$$D\nabla^{4} \mathbf{w} - \frac{1}{\mathbf{r}} \frac{\mathbf{d}}{\mathbf{dr}} \left(\psi \frac{\mathbf{dw}}{\mathbf{dr}} \right) = \mathbf{q} - \frac{1}{1 - \nu} \nabla^{2} \mathbf{M}_{T}$$
 (7)

and

$$\frac{d^2\psi}{dr^2} + \frac{1}{r} \frac{d\psi}{dr} - \frac{\psi}{r^2} + \frac{Eh}{2r} \left(\frac{dw}{dr}\right)^2 = -\frac{d}{dr} N_T$$
(8)

where

$$\nabla^2 = \frac{1}{r} \frac{d}{dr} r \frac{d}{dr}$$

$$\nabla^4 = \nabla^2 \nabla^2 .$$

A more convenient form is obtained by introducing the slope $\beta = \frac{dw}{dr}$, which yields

$$D \frac{d}{dr} r \frac{d}{dr} \frac{1}{r} \frac{d}{dr} (r\beta) - \frac{d}{dr} (\psi\beta) = qr - \frac{1}{1-\nu} \frac{d}{dr} \left(r \frac{dM_T}{dr} \right)$$
(9)

and

$$\frac{d^2\psi}{dr^2} + \frac{1}{r} \frac{d\psi}{dr} - \frac{\psi}{r^2} + \frac{Eh}{2r} \beta^2 = -\frac{d}{dr} N_T.$$
 (10)

Equation (9) may now be integrated with respect to r, resulting in

$$D\left(\frac{d^2\beta}{dr^2} + \frac{1}{r}\frac{d\beta}{dr} - \frac{\beta}{r^2}\right) - \frac{\beta \psi}{r} - \frac{1}{r}\int_{0}^{r} q(\eta) \cdot \eta d\eta - \frac{1}{1-\nu}\frac{d}{dr}M_{T}.$$
(11)

The Eqs. (10) and (11) are to be solved subject to the boundary conditions (obtained from (2) and (4))

(i)
$$\beta$$
 (o) = 0

i) 8(b) prescribed (known slope)

or
$$D\left[\frac{ds}{dr} + \frac{v}{r} B\right]_{r=0}^{+} + \left[\frac{M_{T}}{1-v}\right]_{r=0}^{-} = 0 \text{ (no radial bending moment)}$$
(12)

(iii) $\psi(0) = 0$ (since N_r is finite at r = 0)

(iv)
$$\left[\frac{d\psi}{dr} + \left(\frac{Eh}{b^2 \lambda} - \frac{v}{b} \right) \psi + N_T \right]_{r=b} = 0$$
 (elastic radial restraint "\lambda" at r=b)
$$\left[\frac{d\psi}{dr} - \frac{v}{r} \psi + N_T \right]_{r=b}$$
 prescribed (known radial displacement at r=b)

The above differential equations and boundary conditions can be expressed in non-dimensional form as

$$\ddot{\theta} + \frac{\dot{\theta}}{\bar{r}} - \frac{\theta}{\bar{r}^2} - \frac{\theta \phi}{\bar{r}} = \bar{r} Q(\bar{r}) - \bar{M}_T$$

$$\ddot{\phi} + \frac{\dot{\phi}}{\bar{r}} - \frac{\phi}{\bar{r}^2} + \frac{\theta^2}{\bar{r}} = -\bar{N}_T$$
(13)

where:

$$\vec{r} = \frac{r}{b}, \quad 0 < \vec{r} < 1$$

$$(\cdot = \frac{d}{d\vec{r}})$$

$$\vec{\sigma} = \frac{4b}{b}$$

$$\theta = \frac{b}{b} \sqrt{6(1-v^2)} = 8$$

$$Q(\vec{r}) = \frac{2b^4}{Eh^4} \left[6(1-v^2) \frac{3^{3/2}}{j} \frac{1}{\vec{r}^2} \int_0^{\vec{r}} q_j(\xi) |\xi d\xi| \right]$$

$$\vec{M}_T = \frac{M_T b^2}{Dh} \sqrt{\frac{6(1-v)}{1-v}}$$

$$\vec{N}_T = \frac{N_T b^2}{D} \quad .$$
(14)

The boundary conditions become:

(1)
$$\theta(0) = 0$$

(2)
$$\phi(0) = 0$$

(3)
$$\theta$$
 (1) prescribed or θ (1) + $v\theta$ (1) + \overline{M}_{T} (1) = 0 (14a)

(4)
$$\dot{\phi}(1) + \left(\frac{Eh}{b\lambda} - v\right) \phi(1) + \widetilde{N}_{T}(1) = 0$$
or
 $\dot{\phi}(1) - v \phi(1) + \overline{N}_{T}(1)$ prescribed

The set of nonlinear differential Equations (13) and accompanying boundary conditions (14a) are not amenable to analytic solution. Numerical procedures must be used to obtain solutions for specified values of the parameters. A finite-difference approach is presented below together with numerical results.

4.4 NUMERICAL INVESTIGATION

4.4.1 Finite Difference Procedure

The procedure developed in Reference 4-4 is employed here for the combined thermomechanical problem. In particular, we consider the case of radially immovable edges ($\lambda \to \infty$ in (14a) of Paragraph 4.3.3) with simple supports for bending where q. M_T , and N_T are constant (uniform pressure and temperature which varies only through the thickness). For this problem, (13) and (14a) of Paragraph 4.3.3 reduce to

$$L\theta = \theta \phi + Q \tilde{r}^{2}$$

$$L\phi = -\theta^{2}$$
(1)

and

$$\begin{array}{l} \theta \; (0) = 0 \\ \phi \; (0) = 0 \\ \dot{\theta} \; (1) + \upsilon \; \theta \; (1) + \overline{M}_{\mathrm{T}} = 0 \end{array} \tag{2}$$

$$\dot{\phi} \; (1) - \upsilon \; \phi \; (1) + \overline{N}_{\mathrm{T}} = 0$$

where

$$L = \frac{1}{r} \frac{d}{d\tilde{r}} \frac{1}{\tilde{r}} \frac{d}{d\tilde{r}} \tilde{r}$$

$$Q = \frac{q}{E} \left(\frac{b}{h}\right)^4 \left[6(1-v^2)\right]^{3/2}$$

and the other nondimensional quantities are as defined previously.

A central difference representation of the differential equations (1) yields the following for the interior points:

$$\overline{L} \, \theta_{i} = \theta_{i} \, \phi_{i} + Q \left(\frac{i}{m} \right)^{2}$$

$$(i+1,2,\ldots,m-1,)$$

$$\overline{L} \, \phi_{i} = -\theta_{i}^{2}$$
(3)

where for an arbitrary function ζ ,

$$\bar{L} \left(\zeta_{i}\right) = \frac{i m}{i + .5} \left[(i+1) \zeta_{i+1} - i \zeta_{i} \right] - \frac{i m}{i - .5} \left[i \zeta_{i} - (i-1) \zeta_{i-1} \right]$$
(3a)

m = number of subdivisions

$$\bar{\mathbf{r}}_{i} = \frac{i}{m}$$

$$\frac{1}{0 + \frac{2}{m}} \xrightarrow{i} \frac{1}{m}$$
 $\bar{\mathbf{r}}$

The finite difference form for the representation of the boundary conditions may be written as

$$\theta_{O} = 0$$

$$\theta_{O} = 0$$

$$\theta_{m} = \left[\frac{m\theta_{m-1} - \frac{M_{T}b^{2}}{Dh} \sqrt{\frac{6(1+")}{1-v}}}{(m+v)} \right]$$

$$\theta_{m} = \left[\frac{m\phi_{m-1} - \frac{N_{T}b^{2}}{Dh}}{m-v} \right]$$
(4)

where backward differences are used for the end point $\tilde{\mathbf{r}} = 1$.

Equations (3) and (4) constitute a set of 2(m+1) equations in the 2(m+1) unknowns θ_i , ϕ_i (i = 0,1,2...m). However, these algebraic equations are both coupled and nonlinear, and cannot be solved in closed form. A "related iteration" technique is employed, in which the iterative forms of equations (3) are written as

$$\left[\overline{L}\mathfrak{C}_{\mathbf{i}}\right]_{\mathbf{n}+1} = -\left[\theta_{\mathbf{i}}^{2}\right]_{\mathbf{n}} \tag{5a}$$

$$\left[\overline{L}\theta_{i}^{*}\right]_{n+1} = \left[\theta_{i}\right]_{n} \left[\tau_{i}\right]_{n+1} + Q\left(\frac{i}{m}\right)^{2}$$
(5b)

$$\begin{bmatrix} \theta_{i} \end{bmatrix}_{n+1} = K \begin{bmatrix} \theta_{i}^{*} \end{bmatrix}_{n+1} + \begin{bmatrix} 1 - K \end{bmatrix} \begin{bmatrix} \theta_{i} \end{bmatrix}_{n}$$

$$(i = 1, 2, 3, \dots m-1)$$
(5c)

where K is a "relaxation" parameter.

A typical iteration, starting with the n'th set of iterates $\left[\theta_i\right]_n$ is as follows:

(1) Substitution of [θ_i] into (5a) and making use of the second and fourth of Eq. (4) yieⁱ ds a setⁿ of (m-1) trⁱ-diagonal, linear algebraic equations from which the quantities [ζ_i] are determined.

- (2) Substituting $\begin{bmatrix} \theta_i \end{bmatrix}_n$, $\begin{bmatrix} \phi_i \end{bmatrix}_{n+1}$ into (5b) again yields a set of linear equations from which the provisional values $\begin{bmatrix} \theta_i^* \end{bmatrix}_{n+1}$ for the next set of iterates are determined.
- (3) The actual value of the next set of iterates $\begin{bmatrix} \theta_i \end{bmatrix}_{n+1}$ is obtained from Eq. (5c) where a suitable value of "relaxation" parameter is employed.
- (4) Iterations to convergence are performed.

4.4.2 Numerical Results

Based on the procedure indicated, results are presented in both tabular and graphical form (Table 4.4.2-1 and Figures 4.4.2-1 through 4.4.2-6) for the simply supported solid plate with radially immovable edges.

The nondimensional deflections (\overline{w}) , membrane forces $(\overline{N}_r \text{ and } \overline{N}_t)$, and bending moments $(\overline{M}_r \text{ and } \overline{M}_t)$, presented in the graphs and tables are defined as follows:

$$\bar{W} = \frac{W}{h}$$

$$\bar{N}_{r} = \frac{N_{r}b^{2}}{D}$$

$$\bar{N}_{t} = \frac{N_{t}b^{2}}{D}$$

$$\bar{M}_{r} = -\frac{M_{r}b^{2}}{Dh} \sqrt{6(1-v^{2})}$$

$$\bar{M}_{t} = -\frac{M_{t}b^{2}}{Dh} \sqrt{6(1-v^{2})}$$
(1)

and as defined previously,

$$\frac{r}{\overline{b}} = \frac{r}{\overline{b}}$$

$$\overline{M}_{T} = \frac{Eb^{2}}{\overline{Db}} \sqrt{\frac{\overline{b(1+v)}}{1-v}} \int_{-aTzdz}^{h/2} aTzdz$$

$$\overline{N}_{T} = \frac{Fb^{2}}{\overline{D}} \int_{-aT-z}^{h/2} aTzdz$$

4, 4, 2 (Cont'd)

Stresses (in nondimensional form) may be obtained from these formulas (Reference 4-7):

$$\frac{b^{2}h\sigma_{\mathbf{r}}}{D} = \frac{12z}{h} \left[\frac{\overline{M}_{\mathbf{T}} - \overline{M}_{\mathbf{r}}}{\sqrt{6(1-v^{2})}} \right] + \frac{\overline{N}_{\mathbf{T}}}{1-v} - 12(1+v) \frac{b^{2}}{h^{2}} \alpha T + \overline{N}_{\mathbf{r}}$$

$$\frac{b^{2}h\sigma_{\mathbf{t}}}{D} = \frac{12z}{h} \left[\frac{\overline{M}_{\mathbf{T}} - \overline{M}_{\mathbf{t}}}{\sqrt{6(1-v^{2})}} \right] + \frac{\overline{N}_{\mathbf{T}}}{1-v} - 12(1+v) \frac{b^{2}}{h^{2}} \alpha T + \overline{N}_{\mathbf{t}}$$
(2)

A discussion of the numerical results follows.

(1) Thermal Buckling

The thermal buckling problem of a circular plate due to an average elevated temperature through the thickness (proportional to \overline{N}_T), where $\overline{M}_T=Q=0$ and radial edge displacement is prevented, is equivalent to the mechanical buckling problem of a plate having an edge thrust corresponding to a prescribed edge displacement. It is shown in Reference 4-2 that buckling can occur only when the edge thrust exceeds the lowest eigenvalue of the linearized buckling problem. The critical thrust corresponding to this eigenvalue is given by (Reference 4-5):

$$-\left[\tau_{r}\right]_{cr} = \frac{(2.05)^{2}D}{b^{2}h}$$
 (3)

Since, for the thermal problem, the thrust (up to buckling) is given by

$$\sigma_{\mathbf{r}} = \frac{N_{\mathbf{T}}}{h(1-v)}$$
, (4)

then from (3), (4), and the definition of $\overline{\mathrm{N}}_{\mathrm{T}}$, we find that

$$\left[\overline{N}_{T}\right]_{CT} = 2,94$$

The postbuckling behavior of the plate is given by the nonlinear analysis; the numerical results are presented in Figure 4.4.2-1 and the first five sub-tables of Table 4.4.2-1. The table employs a floating decimal number system which is to be interpreted as shown by the following examples:

$$0.5159E 00 - (0.5159) \times (10^{0}) = 0.5159$$

$$0.5159 \pm 02$$
 $(0.5159) \times (10^2)$ 51.59

$$0.5159E-01 = (0.5159) \times (10^{-1}) = 0.05159$$

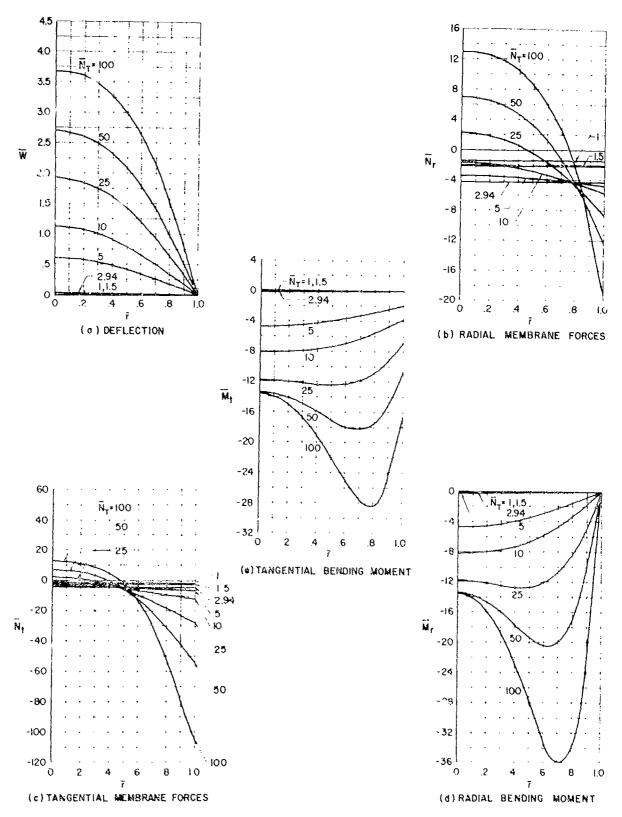


FIGURE 4.4, 2-1 NONDIMENSIONAL THERMAL BUCKLING AND POST BUCKLING BEHAVIOR ($M_{\rm T}$ = Q = 0).

Figure 4.4.2-1 shows the variation of the nondimensional deflection, membrane stress resultants and bending moments with the nondimensional radial coordinate \bar{r} , as \bar{N}_T varies from 0 to 100. It is noted that for $0 < \bar{N}_T < 2.94$ the plate is undeflected, and the two-dimensional linear elastic solution holds, yielding stresses which are constant over the planform and zero bending moments. For higher values of \bar{N}_T , Figure 4.4.2-1a shows nonzero deflections that increase monotonically with increasing \bar{N}_T . For low values of \bar{N}_T , the quantities \bar{N}_T and \bar{N}_t are compressive (Figures 4.4.2-1b and c) throughout the plate. With higher values of \bar{N}_T (25, 50, and 100, for example) deflections in the central region appear to be restrained by tensile membrane stresses, while in the vicinity of the plate edge the membrane stresses become compressive. Figures 4.4.2-1d and e indicate that for small N_T (but above that causing initial buckling), the maximum moments in the radial and tangential directions occur at the center of the plate. With larger \bar{N}_T , maximum moments are away from the center and approach the outer edge as \bar{N}_T increases. The radial moment, in particular, exhibits a strong "boundary layer" effect (sharp gradients near the edge) in meeting the condition $\lfloor \bar{M}_T \rfloor_{\bar{r}=1}^{-1} = 0$.

(2) Bending Due To Combined Thermal And Mechanical Effects

Starting with the sixth sub-table of Table 4.4.2-1, nondimensional numerical results are listed for the thermal bending problem (Q=0), corresponding to the following combinations of thermal parameters:

$$\overline{M}_{T} = 50$$
, 100, 1000, 2000

$$\bar{N}_{T} = 0$$
, 2.94, 5, 10, 25, 100

For a qualitative discussion of the results, we refer to the graphs of Figures 4.4.2-2 through 4.4.2-6 in which mechanical loads $(Q \neq 0)$ as well as temperature are considered.

(a) Deflections (Figure 4.4, 2-2)

For low pressures (quantities proportional to Q) the deflection in the plate interior becomes constant for large temperature differences (quantities proportional to M_T). This flat deflected shape is maintained almost to the plate edges where the bound ry requirement of zero deflection causes a boundary layer effect in which the deflections drop precipitously. This effect becomes less pronounced as the pressure loading increases. However, it is interesting to note that even for high pressure loading (Figure 4.4.2-2d), the plate still tends to flatten out in the central region as the temperature difference increases and, contrary to what would be expected, the deflection at the center does not in all cases increase monotonically with the temperature difference.

(b) Membrane Stresses (Figures 4.4.2-3 and 4.4.2-4)

With increasing temperature difference between the plate faces, the radial and tangential stresses tend to become constant and equal to each other in the plate interior,

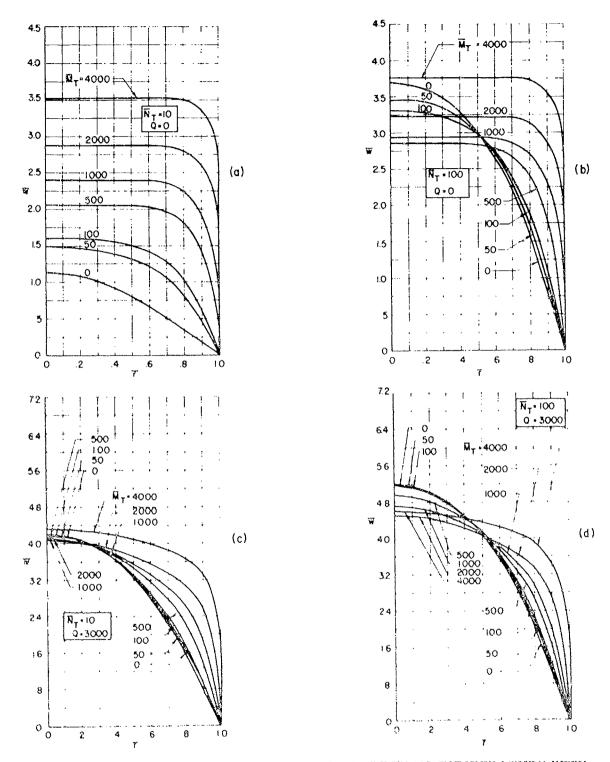


FIGURE 4, 4, 2-2 NONDIMENSIONAL DEFLECTIONS DUE TO TEMPERATURE WITH AND WITHOUT PRESSURE LOADING

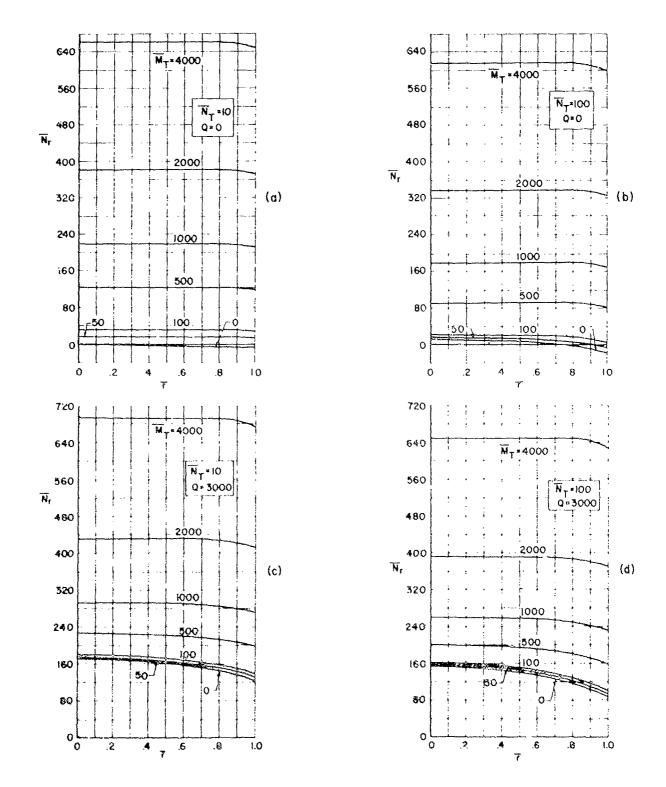


FIGURE 4.4.2-3 NONDIMENSIONAL RADIAL MEMBRANE FORCES DUE TO TEMPERATURE WITH AND WITHOUT PRESSURE LOADING

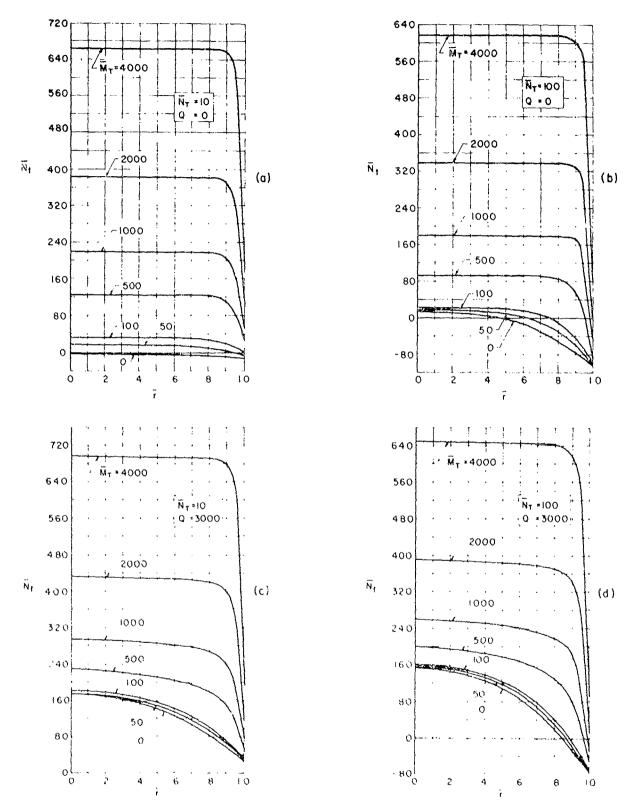


FIGURE 4, 4, 2-4 NONDIMENSIONAL TANGENTIAL MEMBRANE FORCES DUE TO TEMPERATURE WITH AND WITHOUT PRESSURE LOADING

as in a pure membrane. This effect is evident even for high pressure loading, (Figures 4.4.2-3d and 4d). In addition, the magnitudes of these tensile membrane stress resultants increase with increasing temperature difference and normal pressure while they decrease with increasing average temperature (quantities proportional to \overline{N}_T). This is to be expected, since increasing the pressure and temperature difference each cause additional middle plane stretching while \overline{N}_T tends to neutralize this effect. The membrane tension \overline{N}_r , decreases locally as the outer boundary is approached, where the gradients are most pronounced. This negative increment $\Delta \overline{N}_r$, may be responsible for the abrupt reduction in tensile hoop stresses (\overline{N}_t) in the vicinity of the edge (Figure 4.4.2-4). For the larger values of average temperature, the high tensile stress resultants \overline{N}_t , in the plate interior reduce sharply in a boundary layer. Low temperature differences permit transition to compression near the boundary (Figures 4.4.2-4b and d).

(c) Bending Moments (Figures 4.4.2-5 and -6)

The bending moments in the radial and tangential directions are constant and essentially equal over the major central area of the plate. However, radial moments decrease radically near the boundary, satisfying the zero moment boundary condition. Due to the predominantly flat profile of the deflected plate in the interior for large temperature differences (Figures 4. 4. 2-2a and b), it may be conjectured that the constant and equal interior moments are the same as would occur in a clamped plate subjected to the same temperature gradient (since such a plate will remain flat). To show that this is the case, we proceed as follows:

The bending moments in a fully clamped plate subjected to a thermal gradient through the thickness are given by (Reference 4-8)

$$M_r = M_t = -\frac{M_T}{1-v}$$

while

$$\mathbf{w} \equiv 0$$

or, in nondimensional form, using the notation of (14) of Paragraph 4.3.3 and (1)

$$\overline{M}_r = \overline{M}_t = \overline{M}_T$$

This result is readily verified by Figures 4.4.2-5 and -6.

ACKNOWLEDGEMENT

The numerical calculations appearing in this section were performed on the Republic Aviation Corporation IBM 7090 digital computer. The authors wish to express their thanks to M. Gershinsky and B. Sackaroff of the Applied Math. Section, Digital Computing Division for their excellent work in coding and supervising the numerical program.

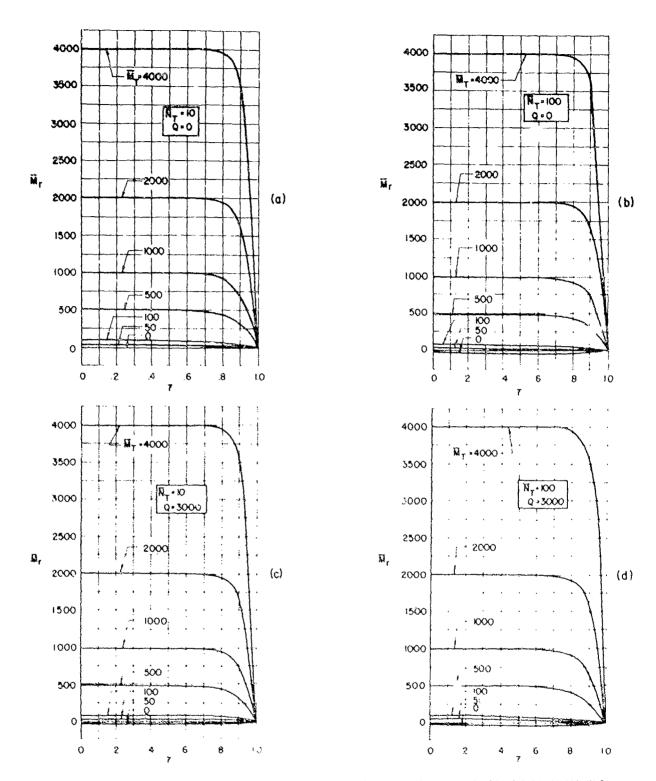


FIGURE 4, 4, 2-5 NONDIMENSIONAL RADIAL BENDING MOMENTS DUE TO TEMPERATURE WITH AND WITHOUT PRESSURE LOADING

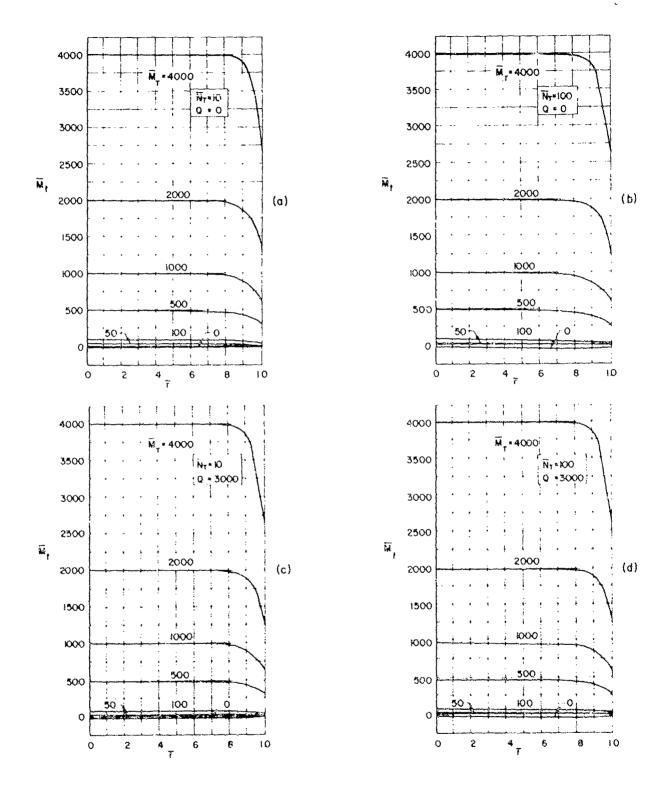


FIGURE 4, 4, 2-6 NONDIMENSIONAL TANGENTIAL BENDING MOMENTS DUE TO TEMPERATURE WITH AND WITHOUT PRESSURE LOADING

TABLE 4.4.2-1

NONDIMENSIONAL DEFLECTIONS, MEMBRANE FORCES, AND BENDING MCMENTS FOR A SIMPLY SUPPORTED CIRCULAR PLATE WITH RADIALLY IMMOVABLE EDGE

(Pages 4, 25 through 4, 35)

TABLE 4.4.2-1 (Cont'd)

$\overline{E}_{\Gamma} = C_{\bullet}$			$N_{T} = 0.29 \text{ Moe}$ Cl			
r	— ₩	Nr	Nt	$\overline{\mathtt{M}}_{\mathtt{r}}$	\overline{h}_{t}	
0.1000E-00 0.2000E-00 0.3000E-00 0.4000E-00 0.4000E-00 0.7000E-00 0.7000E-00	0.2600E-01 0.2506E-01 0.2506E-01 0.2300E-01 0.2077E-01 0.109E-01 0.109E-01 0.109E-02 0.30EE-02	-0.1193E 01 -0.1193E 01 -0.1193E 01 -0.1193E 01 -0.1193E 01 -0.1194E 01 -0.1194E 01 -0.1194E 01 -0.1195E 01 -0.1195E 01	-0.11932 01 -0.11935 01 -0.11945 01 -0.11945 01 -0.11945 01 -0.11945 01 -0.11945 01 -0.11945 01	-0.2072E-00 -0.2013E-00 -0.1962E-00 -0.16124E-00 -0.1612E-00 -0.1612E-00 -0.1612E-01 -0.162E-01 -0.162E-01	-0.2072E-00 -0.2055E-00 -0.2055E-00 -0.106E-00 -0.160E-00 -0.160E-00 -0.13 (1-0 -0.13 (1-0 -0.13 (1-0	
1,00000-00	· · · · · · · · · · · · · · · · · · ·	-0.11%	-0.M100E 1	-0.70/32-07	-(. d.l-31	
	$\mathcal{N}_{K} = \mathcal{N}_{\bullet}$			Ny = 0.430 E	:1	
0.10001-00 0.20008-00 0.30008-00 0.5008-00 0.6008-00 0.70008-00 0.6008-00 0.70008-00 0.90008-00	0.4000 00 0.60000 00 0.6000000 0.6000000 0.60000000 0.60000000 0.600000000	-1.373		-0.0030 1 -0.0000 1 -0.0000 1 -0.0000 0 -0.0000 1 -0.0000 1 -0.0000 1 -0.0000 0 -0.0000 0 -0.0000 0	- 1000 -0.00 H M -0.00 H M -0.00 H M -0.30 H M -0.	
	Try - 1			NT = CORE	15	
0.100E-00 0.100E-00 0.30000 0.400E-00 0.600E-00 0.600E-00 0.500E-00 1.000E-00	0.10 % 01 0.10 % 01 0.10 % 01 0.10 % 01 0.10 % 00 0.00 % 00 0.10 % 00 0.10 % 00 0.10 % 00 0.10 % 00	-0.17% 01 -0.17% 01 -0.17% 01 -0.27% 01 -0.27% 01 -0.37% 01 -0.37% 01 -0.37% 01 -0.37% 01 -0.37% 01 -0.37% 01 -0.37% 01 -0.37% 01 -0.37% 01	-0.1658 31 -0.1678 31 -0.2638 31 -0.2688 31 -0.2688 31 -0.2688 31 -0.74 18 31 -0.268 32 -0.1648 32 -1.1648 32 -1.1648 32	-0.0143E 01 -0.007E 01 -0.007E 01 -0.7082E 01 -0.7082E 01 -0.5507E 01 -0.5507E 01 -0.5507E 01 -0.1657E 01 -0.1657E 01 -0.1657E 01 -0.1657E 01	-0.010 % 01 -0.016 01 -0.006 01 -0.006 01 -0.006 01 -0.006 01 -0.006 01 -0.006 01 -0.006 01 -0.006 01	

4.4.2 (Cont'd) TABLE 4.4.2-1 (Cont'd)

		1ABLE 4.4.4	z i (cont a)		
1 _T = 0.			N _T = 0.2500E 02		
r	$\overline{\mathbf{w}}$	$\overline{\mathtt{N}}_{\mathtt{r}}$	$\widetilde{\mathtt{N}}_{t}$	$\overline{\mathtt{M}}_{\mathtt{r}}$	ři _t
0. 0.1000E-00 0.200D-00 0.3000D-00	0.1936E 01 0.1916E 01 0.1857E 01 0.1759E 01 0.1619E 01	0.2361E 01 0.2250E 01 0.1938E 01 0.1411E 01 0.06031 00	0.2361E 01 0.2039E 01 0.1098E 01 -0.5052E 00 -0.2012E 01	0.1179E 02 -0.118% 02 -0.1011# 02 -0.1012# 02 -0.1266# 02	-0.1179± 02 -0.1101£ 02 -0.1195± 02 -0.1017± 02 -0.1236i 02
0.5000E-00 0.6000E-00 0.7000E-00 0.8000E-00 0.9000E-00	0.11;37E 01 0.1212E 01 0.94;91£ 00 0.6511£ 00 0.32;98E-00 0.	-0.32105 0 -0.15 01 -0.30072 01 -0.4677E 01 -0.6514E 01 -0.8445E 01	-0.5052E 01 -0.9613E 01 -0.1399E 02 -0.1876E 02 -0.2359E 02 -0.2753E 02	-0.1269E 02 -0.1218E 02 -0.1085E 02 -0.5600E 01 -0.533E-05	-0.1245E 02 -0.1032E 02 -0.1182E 02 -0.1070L 02 -0.0149E 01 -0.0355E 01
	$\overline{M}_{T} = 0.$,		N _T = 0.100E	
0. 0.1000E-00 0.20001-00 0.3000E-00 0.4000E-00 0.6000E-00 0.7000E-00 0.8000E-00	0.3684E 01 0.3662E 01 0.3592E 01 0.3468E 01 0.3279E 01 0.2639E 01 0.2151E 01 0.153hE 01 0.7997E 00 0.	0.1306E 02 0.1292E 02 0.1248E 02 0.1170E 02 0.1046E 02 0.8602E 01 0.5893E 01 0.2042E 01 -0.3253E 01 -0.1018E 02 -0.1859E 02	0.1306E 02 0.1263E 02 0.1138E 02 0.8718E 01 0.1373E 01 -0.2625E 01 -0.1350E 02 -0.5210E 02 -0.5210E 02 -0.1055E 03	-0.1377E 02 -0.1377E 02 -0.1577E 02 -0.2751E 02 -0.2751E 02 -0.3774E 02 -0.3601E 02 -0.3617E 02 -0.2771E-04	-0.13%2 02 -0.139% 02 -0.1656E 02 -0.1656E 02 -0.2178E 02 -0.2178E 02 -0.2757E 02 -0.2833E 02 -0.2517E 02 -0.1765E 02
4	0,5000E	0?		$\overline{N}_{T} = 0.$	
0. 0.1000E=00 0.2000E=00 0.3000E=00 0.1000E=00	0.1180E 01 0.1177E 01 0.1166E 01 0.1145E 01 0.1113E 01	0.2166E 02 0.2166E 02 0.216E 02 0.2162E 02 0.2152E 02	0.21662 02 0.21698 02 0.21618 02 0.21518 02 0.21518 02	0.17948 02 0.17738 02 0.1647 E 02 0.1647 E 02 0.1647 E 02	0.1790 00 0.1770 00 0.1770 00 0.1710 0
0.5000E-00 0.5000E-00 0.7000E-00 0.8000E-00 0.9000E-00 1.0000E-00	0.1060E 01 0.081E 00 0.066E 00 0.666E 00 0.666E 00	0.21/3E 02 0.21/3E 02 0.22/E 02 0.20/E 02 0.20/E 02 0.1/27E 02	0.0115a 00 0.006aa 00 0.1963a 00 0.1757a 00 0.1333a 00 0.5783a 01	0.10.000 on 0.3.57 to on 0.3.57 to on 0.0.000 on 0.10.000 on 0.21.00001	0.1860. 10 0.3766. 0 0.3766. 0 0.3766. 0 0.1860. 10

4.4.2 (Cont'd) TABLE 4.4.2-1 (Cont'd)

- M _T = 0.5000E 02				M _T = 0.89h0E 01			
r	w	$\overline{\mathtt{N}}_\mathtt{r}$	− N _t	Fir	- Fit		
0.	0.1270E 01	0.2027E 02	0.00275 02	0.11760E 02	0.1760± 02		
0.10005-00	0.12666 01	0.2027E 02	0.2026E 02	0.1171.4ie 02	0.1.751E 02		
0.2000出-00	0.125h£ 01	0.2025E 02	0.2021E 02	0ء غيا69 ما 02	0.47221 02		
0.3000E-00	0.12308 01	0.2022E 02	0.2012E 02	0.4603E 02	0.46711 02		
0.40006-00	0.1193E 01	SO 38108*0	0.1795E 02	0.445ha 08	0.1590E 02		
0.50004-00	0.1136E 01	0.20105 02	0.12621 02	0.422hib 02	0.14,682 02		
0.60P0E-00	อ.โอโดย 01	0.10981 02	0.1.995 02	0.3875E 02	0.4288: 02		
0.70001-00	0.9187E 00	0.12765 02	0.17785 02	0.3352± 02	0.40850-08		
O_€00005=00	0.7239E 00	0.19385 02	0.15324 (2	0.2575b 02	0.3611 02		
∂ , 90000 L= 00	0.li330b=00	0.18715 02	J.1060E U2	0.11.382 02	0 , 30984 02		
10000E-00	0.	0.17h7b 02	0.2329± Ol	-0.7152E-05	0.2385L 02		
hip = 0.5000E 02		$N_{\rm T}$ = 0.5000E O1					
0.	0.133/E 01	0.1943E 02	0.1913E 02	0.47368 02	0,117363 02		
0.10005-00	0.1330E 01	0.1955 05	O.IMIE (I	0.4718E 00	0.1.7202 02		
0.2000E=00	0.131cb 01	$0.15\mu 07.05$	0.1936E 07	0*J1000E 03	O. hoson of		
0.3000E=00	0.12/1E 01	0.177E 03	0*1352E 05	J*F250F 05	O* (164) JT 95		
0.11300100	0.12502 01	0.10301.00	0.190hg 00	०.मिताइट २१	0.45576 00		
J.5000H=00	0.1138E 01	0.19% 55 02	d. Leool, ue	0.h178E 02	0*jql3T2 :.5		
0.6 J.C00	0.10개位 01	0.19083 OC	0.179hE 02	J. 30, 12 05	Ocholica or		
0.7.000	0.956IE 00	0.10030 00	0.1057. 00	90°35 901°95	0.39752 00		
0.40001.400	0.7507E 00	0.18405 02	0.1390E 00	0.03737 05	0.350/17 00		
9.00000000	0.hh72E-00	0.17652 02	IV 11/578.6	り・チょうしゃ つち	0.30557 05		
1.000-00	Ŭ•	0.1631E 02	-0.6811E-01	-0.117.602-06	0.23558 02		
Enth = O = Colon Os		Tight = O.T.C.SI OS					
().	11, 202 71	0.1 200 00	0.17762 02	0.16758 02	0.16758 02		
$()()=,[(\ ()()\]_{\bullet}()$	0.1h8h: 01	0.17758 02	0.177182 02	J*PORRE 05	0.46635 02		
0.50000000	0.16676 01	0.17736 02	0.1706E 02	0.467年 02	0.46306 02		
()*3')()()***()()	0.11368 CI	ONTROOM OF	0.17496 00	0*PhoOn 05	0.4530E 95		
0.4000E-00	0.1388E 01	0.17602 02	0.17205 08	0.113010 08	0。14477年 62		
0.500000	0.131/12/01	0.171711 02	0.16678 02	0.4066年 02	0.43h0E 02		
0.60003-00	0.150gg 01	0.1727E : 0	0*3293E 05	0,36092.02	0.41438 02		
0.7000000	$0.10 \mathrm{hor}$ 01	0.16936 02	0.1390% 02	0.31438 02	0°389PE 05		
9. (30) XJE -(30)	0 . 81550 00	0.16365 02	O. TOURE OF	0.036/15 00	0. 317/1000		
·,*\io\()(\)''*()()	0.かにょっ!!=00	०,गडीवाह ००	O*P35RE OI	0.1283E 02	0.50/ITV 05		
	0.	0.1378E ON	-0.58250 ol	-0.81065-05	0.02000 00		

4.4.2 (Cont¹d)

TABLE 4.4.2-1 (Cont'd)

Fig = 0.5000E 02			$\overline{N}_{T} = 0.2500E O2$			
r	w	$\overline{\mathtt{N}}_{\mathtt{r}}$	$\overline{\mathtt{N}}_{\mathtt{t}}$	$ar{\mathtt{M}}_{\mathtt{r}}$	i î _t	
0. 0.1000E-00 0.2000E-00 0.3000E-00 0.1000b-00 0.5000E-00	0.1936E 01 0.1928E 01 0.1902E 01 0.1055E 01 0.1763E 01 0.1676E 01 0.1522E 01	0.1510E 02 0.1508E 02 0.1508E 02 0.1502E 02 0.1h91E 02 0.1h46E 02 0.1h02E 02	0.1510± 02 0.150h± 02 0.1h86± 02 0.1h49± 02 0.136h± 02 0.1273± 02 0.1002± 02	0.4498E 02 0.4498E 02 0.4261E 02 0.4261E 02 0.4051E 02 0.3311E 02	0.1498± 02 0.1483± 02 0.1440± 02 0.1363± 02 0.121;7± 0.1;080± 02 0.3618± 02 0.3537± 02	
0.00005-00 0.00005-00 0.00005-00	0.130%E 01 0.100LE 01 0.47.72 00	0.13354 00 0.1030E 00	0.7521E 01 0.1813E 01 -0.801E 01 -0.2051E 02	0.2719E 02 0.1939E 02 0.7722E 01 -0.1237E-04	0.31313 02 0.3625E 02 0.207hE 02	
	0.5000€ 00			W _p = 0.10008		
0. 0.10004-00 0.10004-00 0.30004-00 0.40004-00 0.8004-00 0.8004-00 0.90004-00	0.368 % of 0.34760 of 0.34760 of 0.33400 of 0.3036 of 0.3036 of 0.3036 of 0.37324 of 0.47324 of 0.49866 oo	0.1786.02 0.1679E 07 0.1679E 07 0.1679E 07 0.180E 07 0.180E 07 0.1870E 07 0.1070E 07 0.1071E 01	0.160.01 00 0.160.01 00 0.160.00 00 0.160.00 00 0.160.00 00 0.160.00 00 0.160.00 00 0.160.00 00 0.160.00 00 0.160.00 00	0.1035E 00 0.3772E 00 0.369E 00 0.369E 00 0.3100E 00 0.6330E 01 -0.1030E 01 -0.6317E 01 -0.2061E-05	0.10534 02 0.10594 02 0.35774 02 0.35776 02 0.35186 02 0.22806 02 0.22806 02 0.17076 02 0.12314 02	
0. 0.1000L-00 0.1000L-00 0.3000E-00 0.4000E-00 0.0000E-00 0.8000E-00 0.9000E-00	0.1380E 01 0.1380E 01 0.138E 01 0.136E 01 0.136E 01 0.130E 01 0.130E 01 0.130E 01 0.236E 00 0.596E 00	0.37126 00 0.37126 00 0.37126 00 0.37146 00 0.37146 00 0.37136 00 0.37036 00 0.3080 00 0.36576 00	0.37106 08 0.37106 08 0.37036 08 0.37036 08 0.37036 08 0.37036 08 0.37036 08 0.37036 08 0.37036 08	0.00016 00 0.00016 00 0.00016 00 0.00016 00 0.00016 00 0.00016 00 0.00016 00 0.00016 00 0.00016 00	0.08012 02 0.08012 02 0.08136 02 0.08136 02 0.08136 02 0.08136 02 0.08136 02 0.71118 02 0.83026 02 0.83026 02	

4.4.2 (Cont'd) TABLF *.4.2-1 (Cont'd)

M _r = 0.1000E 03			N _T = 0.2940E Ol		
ī	$\overline{\mathbf{w}}$	\overline{N}_r	$\overline{\mathfrak{r}}_{t}$	$\overline{\underline{M}}_{r}$	\overline{A}_{t}
0. 0.1000E-00 0.2000E-00 0.3000E-00 0.6000E-00 0.7000E-00 0.8000E-00 0.9000E-00	0.1450E 01 0.1448E 01 0.1448E 01 0.1498E 01 0.1363E 01 0.1293E 01 0.1170E 01 0.9661E 00 0.6110E 00 0.	0.3557E 02 0.3557E 02 0.3556E 01 0.3556E 01 0.357E 02 0.357E 02 0.357E 02 0.3497E 02 0.3497E 02 0.3497E 02 0.3420E 02	0.3557E 02 0.3557E 02 0.3552E 02 0.3552E 02 0.3555E 02 0.3555E 02 0.3555E 02 0.3565E 02 0.376E 02 0.2376E 02 0.7190E 01	0.9876E 02 0.9861E 02 0.9861E 02 0.9551E 02 0.9551E 02 0.3753E 02 0.7880E 02 0.6371E 02 0.3795E 02 0.1496E-02	0.9876E 02 0.9867E 02 0.9840E 02 0.9769E 02 0.9551E 02 0.9501E 02 0.8891E 02 0.8202E 02 0.7052E 02 0.5319E 02
M,	r = 0.1000E 0	3		$\overline{N}_{T} = 0.5000E$	01
0. 0.1000E-00 0.2000E-00 0.3000E-00 0.4000E-00 0.5000E-00 0.6000E-00 0.8000E-00 0.9000E-00	0.1495E 01 0.1493E 01 0.1485E 01 0.1471E 01 0.1446E 01 0.1407E 01 0.1326E 01 0.1207E 01 0.9886E 00 0.6237E 00	0.3462E 02 0.3461E 02 0.3460E 02 0.345EE 02 0.345E 02 0.345E 02 0.345E 02 0.339E 02 0.3312E 02 0.3316E 02	0.31,62E 02 0.31,60E 02 0.31,60E 02 0.31,56E 02 0.31,56E 02 0.31,29E 02 0.33,82E 02 0.32,67E 02 0.27,71E 02 0.21,28E 02 0.41,731,45 01	0.9867E 02 0.9851E 02 0.9801E 02 0.9703E 02 0.9703E 02 0.9707E 02 0.7707E 02 0.77190 02 0.7303E 02 0.37352 02 0.2555E-03	0.9867E 02 0.9858E 02 0.9558E 02 0.9776E 02 0.683L 02 0.9569E 02 0.057E 02 0.157E 02 0.707E 02 0.5293E 02
$\overline{M}_{T} = 0.1000E 03$			N _T = 0.1 100E	02	
0. 0.1000E-00 0.2000E-00 0.3000E-00 0.1000E-00 0.5000E-00 0.6000E-00 0.7000E-00 0.8000E-00 0.9000E-00	0.160hE 01 0.1600E 01 0.1593E 01 0.1576E 01 0.15h7E 01 0.1h97E 01 0.1h1hE 01 0.1275E 01 0.10h2E 01 0.6523E 00	0.3253E 02 0.3253E 02 0.3253E 02 0.3254E 02 0.3254E 02 0.3234E 02 0.3234E 02 0.3213E 02 0.3170E 02 0.3072E 02	0.3253E 02 0.3253E 02 0.3251E 02 0.3256E 02 0.3209E 02 0.3009E 02 0.2660E 02 0.178hE 02 -0.100hE 01	0.98h2E 02 0.982hE 02 0.9769E 02 0.91h5E 02 0.91h5E 02 0.7678E 02 0.61h1E 02 0.3598E 02 0.5135E-04	0.9831E 07 0.9831E 07 0.9800E 07 0.9740E 07 0.9639E 07 0.9763E 07 0.8763E 07 0.6901E 07 0.6901E 07

4.4.2 (Cont'd)

TABLE 4.4.2-1 (Cont'd)

$\overline{M}_{T} = 0.1000E 03$						
r	 W	n _r .	$\overline{\mathtt{N}}_{t}$	$\overline{\mathbb{T}}_{\mathbf{r}}$	$\overline{\mathtt{M}}_{t}$	
0.	0.193度 01	0.2797L 02	0.2797£ 02	C. 7756£ 02	0.97562 02	
9 .1 000£-09	0.1930E 01	0.27.964-02	O.2795E 02	0.1733¥ 02	0.97b3E 02	
0 , 2000U=00	0.1917E 01	0.2795E 02	0.27904 02	0.7601E 02	0.9702E 02	
0.30001-00	0.18925 01	0.27925 02	0.27792-02	O。9521起 G2	0.7626ь 02	
0.4000E-00	0.18500 01	0.2786E 02	0.2756L 12	0°3563F 05	0.9500E 02	
0.5000E-00	0.1790± 01	0.2776⊾ 02	0.27084 02	0.69024 08	0.2299年 02	
0.6000E-00	0.16696 01	0.2757± 02	0.26015 02	0.02773 02	0.8985E 02	
0.7000E-00	0.14896 01	0.2720E 02	0.23666 02	0.7273년 02	0.84961 02	
0.8000H-00	0.1300% 01	O*SQP()E US	0.18318 00	0.5630F 05	0.77hib 02	
0.9000E-00	0.7377E 00	O*57733E O5	0.60613 01	0.3213E 0?	0.6595± 02	
1.。0000世-00	ી.	0.2189E 02	-0.1798E 02	-0.2350B-0h	0.50453 02	
74	r = 0.1000k 03	3		$\vec{N}_{1} = 0.1000E$	03	
0.	0.3313E 01	0.2268E 02	0.2268E 02	0.9110E 02	0. श. १ व २००	
0.1000a-00	0.33035 01	0.2366E 03	O.2260E 02	0.9365E 02	o. 1381æ or	
0.50005~00	0.3515E OT	0.2057E 02	O.2232E 02	0.9227E 02	0*302E 05	
0.3000L-00	o.gorka or	O'SEPOR OS	0.517/hr 05	0.8971E 02	0.91638 02	
○ ¹¹```\``\`\`\`\`\`\\\\\\\\\\\\\\\\\\\\	0.31102 01	O'SSITE US	0.2059£ 02	O.8550a O2	0.8934此 92	
0.50002-00	0.2970201	0.2161E 02	0,185úH or	0.1003E 75	0.8590E 02	
J*6000E=00	9.534Ta 9J	0.2075E 02	0.13953 02	0.0/32E 02	0.8086E 0?	
0.70006-00	0.0373 01	0.1003E 02	0.5233E 01	0.54368-02	0.7371正 02	
0.80008-	1.1720 01	0.16535-02	-0.1201E 02	0.36618 02	0.引用器 65	
0.900001-00	`.1101E 01	0.1172E 08	-0.1537E 02	0.0500×0.05	0.5270E 02	
1.00008-00	;).	0.3266E 01	-0.9855E 02	~0.381hE=0h	0.452QE 05	
- ! *]	, = O.J.OOE Oh			$\overline{N}_{T} = 0$.		
(1.	व अविद्याल	0.22568.03	0.55 JE 03	[*()():) ⁽⁾ E; 13	1.00002 03	
O_{\bullet} 1000 P_{\bullet} 00	الله المالية وقول	0.22400.03	0.22461, 03	1*0000P 93	1.00006 03	
0.50000=00	1,602, 20 CT	200 at 03	0.27/66 03	0^{\bullet} second 03	0.0000p. (3	
J. J. J. J. J. J.	ા•ુ≎ત્રાહું∴ા1	9.00 Port 13	7.02 hot: 03	() _• >>ə\;\;\;\;\;\;\;\;\;\;\;\;\;\;\;\;\;\;\	()*0000E 93	
() * \f()()()! *-();)	0.53 PET 01	0.30 ion 03	O*55,PèE 03	O. Marka 33	0,000,001, 13	
$J^{\bullet}(i) J(J_i) = C_i(J_i)$	0.8343F GT	0.00 Mos 03	0.2546r 63	0.00000 33	ເ) • າປຕິດພົ <i>້</i> ກັ	
9.0000E=00	J*03320 J	0.00 per 03	0.22) jšk. už	0.20628 63	0.08H. 03	
1.30 (0.10.1-0.)	0.03136.01	- 0 . 00/6/E-03	0*55 Phile 63	0.00130 03	0.03300.03	
()*{}'/()()[]=()e)	C.PARRET	0.03 juli 3	0 . 223jir 03	0.13/10/103	9.9485E 03	
$(J^{\bullet} \circ)(J)[J]^{\bullet}(J')$	TO HISTOR	0.50 Pr. 03	0.21.07n 03	9 13	0^{\bullet} goods -33°	
100006-00	().	0.0170E 03	0.6505E 02	0.531/18 00	0.62893 03	

4.4.2 (Cont'd) TABLE 4.4.2-1 (Cont'd)

$\overline{M}_{T} = 0.1000E$ Oly			N _T = 0.2940E 01		
r	w	$\overline{\mathtt{N}}_{\mathtt{r}}$	$\overline{\overline{N}}_{\mathbf{t}}$	$\overline{\mathbb{N}}_{\mathbf{r}}$	$\overline{\kappa}_{t}$
0.	0.23621 01	0.2229£ 03	0,2229E 03	1.0000E 03	1.0007E 03
0.1000E-00	0.2362E 01	0.2229E 03	0.2229E 03	0.9999E 03	1.000001 03
0.2000E-00	0 , 2362E 0 1	0.2229E 03	0,2229E 03	O•1799E 03	0.9999E 03
0.3000E-00	0 . 2361E 01	0.2229E 03	0.2229E 03	0.9999E 03	0.99992 03
0.4000E-00	0.23611 01	0.2229E 03	0.2229E 03	0 . 9997E 03	0.9998E 03
0.5000E-00	0.2360E 01	0.2229E 03	O•2229E O3	0.9990E 03	0 . 9996E 03
0.6000E-00	0.2351年 01	0,2229E 03	0.2229E 03	0.9961 <u>5</u> 03	0.7984E 03
0.7000E-00	0.2329E Ol	0.2229E 03	O.2228E 03	0.9840E 03	0.99 <u>35</u> £ 03
0.8000E-00	0.2226E 01	0°3558E 03	0.22202 03	0•73 كار 39•03	0.97490 03
0.9000E-00	0.1797E Ol	0.2023E 03	0.2090E 03	0.72.15± 03	0.82752 03
1.0000E-00	0.	0.2153E 03	0.6208E 02	0.1335E-01	0.6285E 03
	r = 0.1000E ol		0,2217£ 03	N _T = 0.5000E	01 1.0000E 03
0.7.0000 00	0.2373E Ol	0.22178 03	· · · · · · · · · · · · · · · · · · ·	0.9999E 03	1.0000E 03
0.1000E-00	0.23735 01	0.2217E 03 0.2217E 03	0.2217E 03 0.2217E 03	3000F 03	0.9999E 03
0.2000E-00 0.3000E-00	0.2373E 01 0.2373E 01	0.2217E 03	0.2717E 03	0.4999E 03	0.999 E 03
0.1000E-00	0.2373E 01	0.27 t/E 03	0.22171 03	0.9997E 03	0.9998E 03
0.5000E=00	0.23718 01	0.20178 03	0.2217E 03	0.9990E 03	0.79968 03
0.5000E=00	0.23656 01	0.2217E 03	0.2.2178 03	0.99:0E 03	0.4981正 03
0.7000E-00	0.23406 01	0.22171 03	0.2217E 03	0.9838E 03	0.9937E 03
0.8000E-00	0.22365 01	0,22175 03	0.2208E 03	0.9330E 03	0.97/17L 03
0.9000E-00	0.180hE 01	0.2212E 03	0.2076E 03	0.7206E 03	0.8.71E 03
1.0000E-00	().	0.21hIE 03	0.59811 02	0.3875E-02	0.628lp 03
M _T = 0.1000E oh			N _T = 0.1000E	02	
0.	0.Shore or	0.2191E 03	0.2191E 03	1.00001 03	1,0000E 03
0.1000E-00	o.ehore or	0.2191E 03	0.2191E 03	0.9999E 03	1.0000E 03
0.2000E-00	o. Shore of	0.219TE 03	0.2191E 03	0.99991 03	0.9999E 03
0.3000E-00	0.2hole of	0.2191E 03	0.2191E 03	0.9999E 03	0.9999E 03
0.4000E-00	0.21:013 01	0.2191E 03	0.2191E 03	0.9997E 03	0.9998E 03
0.5000E-00	0.2399E OL	0.2191E 03	0.2191E 03	0.9990E 03	0,9995E 03
0.6000E-00	0.2393E 01	0.2191E 03	0.2191E 03	0.9959E 03	0.9983E 03
0.7000E-00	0.2367E 01	0.2191E 03	0.2190E 03	0.983hE 03	0.9935E 03
0.8000E-00	0.2260E 01	0.2190E 03	0.2181E 03	0.9318E 03	0.9743E 03
0.9000E-00	O*1850E OI	0.2185E 03 0.2112E 03	0.20HH 03 0.5386E 02	0.7182E 03 0.1136E-02	0.8961E 03 0.6280E 03
1.0000F00	0.	Umallab Uj	ひゅうしいい ひん	ひょよよりひかーひと	U DECUCE US

4.4.2 (Cont'd) TABLE 4.4.2-1 (Cont'd)

$\overline{M}_{T} = 0.1000E Oh$			$N_{T} = 0.2500E 02$			
r	$\widetilde{\mathtt{w}}$	N _r	\overline{N}_{t}	Mr	$\overline{\mathtt{M}}_{t}$	
, O.	0,2486E 01	0.2114E 03	0.211l± 03	1.0000E 03	1.0000E 03	
0.1000E-00	0.2486E 01	0,211\iE 03	0.211lib 03	0.9299E 03	0.9999世 03	
0.2000E-00	0,2486E 01	0.211世 03	0,2114E 03	0.9999년 03	0.9999E 03	
0.3000E-00	0.2486E 01	0.211lE 03	0.211LE 03	0.2929E 03	0.9999E 03	
0.4000E-00	0.24862 03	0.21148 03	0.2111 منابا 03	0.9997E 03	0.9998E 03	
0.5000E-00	0.2486E 01	0.2111 03	0.211hE 03	0.9988E 03	0.999511 03	
0.6000E-00	0.2477E 01	0.211LE 03	0.21135 03	0.29551 03	0.0981= 03	
0.70002-00	0.214,88 01	0.2113E 03	0.2113E 03	0.98218 03	0.29301 03	
0.80001-00	0.23,26 01	0.2113E 03	0.2102E 03	0.9283E 03	0.9728E 03	
0.9000E-00	0.1869E 01	0.2107E 03	0.19521 03	0.7113E 03	0.8932E 03	
1.0000E-00	0	0.2029E 03	0.3657E 02	0.721:7E-03	0.6258E 03	
M ₋	r = 0.1000E ol	0,1791£ 03	0.17918 03	0,7139E 03	03 (<u>) 2020 E (3</u>	
0.1000E-00						
	0.2925E 01 0.2925E 01	0.17위표 03	0.17分配 03	0.4 20年 23	0 ° 300 35 ′33 ′	
0.2000E-00 0.3000E-00	0.2925E OI	0.179进 03	0.179度 03	0.1157 93	0.000 33	
0.1t000E=00	0.5857m 01	0.179底 03 0.179底 03	0.17위표 0) 0.17위표 03	0°330[F 33	0.00/E 03 0.00/E 03	
0.5000E-00	0.2920E 0I	0.179位 03 0.179位 03	0.17 Mile 03	9.33507.93	(1) (1) (1) (1) (1) (1) (1) (1) (1) (1)	
0.6000100	0.2907E OL	0.179症 03	0.179世 03	5 335 63 6 375 63	2 3 1 1 1 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	
0.7000E-00	0.006E 01	0.179位 03 0.179位 03	0.1792E 03	0.34k(08.93	1000F 03	
0.8000E-00	0.2006E GI	0.1793E 03	0.1772E 03	0. 10% 03	0. 205 (1) 3	
0.9000E-00	0.2111E 01	0.1783E 03	0.1536E 03	6*\dagge 03		
J.0000E-00	().	0.10702 03	-0.10.4E 05	A CONTRACT	(1) (1) (1) (1)	
£ • (/ / / / / / / / / / / / / / / / / /	~ · ·	O & D (1 C/G)	-Oatte Mill Of	4.1. () - 1		
<u> </u>	r = 0.2000E 0			$\overline{N}_{T} = O_{\bullet}$		
() *	० १६३।। ध	०,३६७/मः ७३	0, 367/16, 13	0.0000000000000000000000000000000000000	• • • • • • • • • • • • • • • • • • • •	
()*J();QE='()()	०,०७३५ अ	ं अभूति । ।	0. 3,,5, TO 03	المناء البراران فورا	1. O 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	
()()()()()()—()()	0.317 01	ा अध्यक्षित अ	0.3 74/2 -13	Got and off	• • • • • • • • • • • • • • • • • • • •	
J 3000 -00	0 % 312 01	1, 31, 111 13	9,36,30,30	100 11 1		
: 1000E-00	9.5 31E 9I	り。32月底(3	7*355.3E		• 2 2 2 2	
0.2000E=00	0.09.48.01	0.36738 03	3. 19735 3	1 1 to 1	1:	
1,*()()()()=()()	0.20302 1	0. 877E 03	O 433 30 33	•1 / 4 \h.		
$() \bullet \lambda () (x, y, \neg () ()$	o.aan d	0.38738-03	ं, उहर्रह्म छ	- C.1 (AL) - L		
() • (3()()()E=()()	0.27688 01	0.3633E 03	% 3000E 13	11. 1. 11. 12 1 1 1 1 1 1 1 1 1 1 1 1 1	1.1 11 1.	
0.9000E-00	Collocation of	0.3870E 03	3, \$766E 03	16377		
1.0000£-00	().	0.37/82 03	.17003	0.77.01. 17	1. (1.	

4.4.2 (Cont'd)

TABLE 4.4.2-1 (Cont'd)

$\overline{M}_{\Gamma} = 0.2000E \text{ Ol}_{4}$			$\overline{II}_T = 0.29140E \text{ Ol}$			
ī	w	$\overline{\mathtt{N}}_{\mathtt{r}}$	$\overline{\mathtt{N}}_{\mathtt{t}}$	~ Mr	Mt	
0. 0.1000E-00 0.2000E-00 0.3000E-00 0.4000E-00 0.5000E-00 0.6000E-00 0.8000E-00 0.9000E-00	0.2842E 01 0.2842E 01 0.2842E 01 0.2842E 01 0.2842E 01 0.2841E 01 0.2832E 01 0.2778E 01 0.2416E 01 0.	0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03	0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3856E 03 0.3654E 03 0.3750E 03	0.2000E O4 0.2000E O4 0.2000E O4 0.2000E O4 0.1999E O4 0.1998E O4 0.1991E O4 0.1914E O4 0.1631E O4	0.2000E OU 0.2000E OU 0.2000E OU 0.2000E OU 0.2000E OU 0.1999E OU 0.1999E OU 0.1996E OU 0.1980E OU 0.1870E OU 0.1286E OU	
$\overline{M}_{T} = 0.2000E Oli$			N _T = 0.5000E 01			
0. 0.1000E-00 0.2000E-00 0.3000E-00 0.4000E-00 0.5000E-00 0.6000E-00 0.8000E-00 0.9000E-00	0.2850E 01 0.2850E 01 0.2850E 01 0.2850E 01 0.2850E 01 0.2850E 01 0.2860E 01 0.2860E 01 0.2785E 01 0.2421E 01	0.3845E 03	0.3815E 03	0.2000E 04 0.2000E 04 0.2000E 04 0.2000E 04 0.1999E 04 0.1991E 04 0.1991E 04 0.191E 04 0.1630E 04	0.2000E 04 0.2000E 04 0.2000E 04 0.2000E 04 0.2000E 04 0.1999E 04 0.1999E 04 0.1990E 04 0.1870E 04 0.1870E 04	
M.	r = 0.2000E of	ļ		- N _T = 0.1000E	02	
0. 0.1000E-00 0.2000E-00 0.3000E-00 0.4000E-00 0.5000E-00 0.7000E-00 0.9000E-00	0.2869E 01 0.2869E 01 0.2869E 01 0.2869E 01 0.2869E 01 0.2869E 01 0.2867E 01 0.2859E 01 0.2803E 01 0.21, 44E 01 0.	0.3817E 03 0.3817E 03 0.3817E 03 0.3817E 03 0.3817E 03 0.3617E 03 0.3617E 03 0.3817E 03 0.3817E 03 0.3817E 03	0.3817E 03 0.3817E 03 0.3817E 03 0.3817E 03 0.3817E 03 0.3817E 03 0.3817E 03 0.3817E 03 0.3814E 03 0.3708E 03 0.1025E 03	0.2000E 04 0.2000E 04 0.2000E 04 0.2000E 04 0.2000E 04 0.1999E 04 0.1998E 04 0.1991E 04 0.1943E 04 0.1627E 04	0.2000E OL 0.2000E OL 0.2000E OL 0.2000E OL 0.1999E OL 0.1999E OL 0.1999E OL 0.1979E OL 0.1869E OL 0.1286E OL	

 $\overline{M}_{T} = 0.2000E 04$

 $\overline{N}_{T} = 0.2500E 02$

r	¥	Nr	₹ _t	$\mathcal{H}_{\mathbf{r}}$	¥t
0.	0.2926E 01	0.3738E 03	0.3738E 03	0.2000E 04	0.2000E 04
0.1000E-00	0.2926E 01	0.3738E 03	0.3738E 03	0.2000E 04	0.2000E 04
0.2000E-00	0.2926E 1	0.3738± C3	0.3738E 03	0.2000E 04	0.2000E 04
0.3000E-00	0.2926E 01		0.3738E 03	0.2000E 04	0.2000E 04
0.4000E-00	0 26E 01	C.3738E 03	0.3738E 03	0.2000E 04	0.2000E 04
0.5000E-00	0.2925E 01	O.3738F 03	0.3738E 03	0.1999E 04	0.1999E 04
0.6000E-00	0.2924E 01	0.37381.03	0.3738E 03	0.1998E 04	0.1999E 04
0.7000E-00	0.2915E 01	0.37 JE 03	0.3738E 03	0.1990E 04	0.1996E 04
0.8000E-00	0.2855E 01	0.3 38E 13	0.3734F 03	0.1941E 04	0.1979E 04
0.9000E-00	0.2473E 01	0.3734E 03	0.362CJ 03	0.1620E 04	0.1866E 04
1.0000E-00	0.	0.3626E 03	0.8473E 02	0.5188E-03	0.1285E 04

M̄_T ≈ 0.2000E 04

 $\overline{N}_{T} = 0.1000E 03$

0.	0.3215E 01	0.3378E 03	0.3378E 03	0.2000E Ol	0.2000E 04
0.1000E-00	0.3215E 01	0.3378E 03	0.3378E CJ	0.2000E OL	0.2000E 04
0.2UJUE-00	0.3215E 01	0.3378E 03	U.3378E 03	0.2000E 04	0.2000E 04
0.3COOE00	0.3215E 01	0.3378E 03	0.3378E U3	0.2000E 04	0.2000E 04
0.4000E-00	0.3215E 01	0.3378E 03	0.3378E 03	0.1999E OL	0.2000E 04
0.5000E-00	0.3215E 01	0.3378E 03	0.3378E 03	0.1999E Ol	0.1999E Ol
0.6000E-00	0.3213E 01	0.3378E 03	8E J3زو، 0	0.1997E OL	0.1999E 04
0.7000E-00	0.3199E 01	0.3378E 03	0.3378E 03	0.1988E 04	0.1995В ОЦ
00-2000E-00	0.3122E 01	0.3378E 03	0.3373E 03	0.1929E 04	0.1974E Ol
0.9000E-00	0.2669E 01	0.3373E 03	0.3216E 03	0.1587E 04	0.1853B OL
1.0000E-00	0.	0.3242E 03	-0.9233E 00	0.2807E-02	0.1280E 04
0.8000E-00 0.9000E-00	0.3122E 01 0.2669E 01	0.3378E 03 0.3373E 03	0.3373E 03 0.3216E 03	0.1929E 04 0.1587E 04	0.197LE OL 0.1853E OL

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SECTION 5

AXISYMMETRIC STRESSLS AND DEF ECTIONS IN . LLS DUE TO THEE MAL AND MUCHA ICAL LOADS

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M. Forray

SECTION 5

AXISYMMETRIC STRESSES AND DEFLECTIONS IN SHELLS DUE TO THERMAL AND MECHANICAL LOADS

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SECTION 5 - AXISYMMETRIC STRESSES AND DEFLECTIONS IN SHELLS

DUE TO THERMAL AND MECHANICAL LOADS

5.1 SUMMARY

This section considers the axisymmetric stresses and deflections in heated shells under load. Basic equations, applicable to general shells of revolution, are derived in Sub-section 5.3. These are discussed and applied specifically to conical shells (Sub-section 5.4) and cylindrical shells (Sub-section 5.5) where solutions to the differential equations are given.

5. 2. INTRODUCTION

The purpose of this section is to present the theory for the linear elastic analysis of shells of revolution subjected to axisymmetric mechanical loads and temperatures. The formulation used is the linearized version of the development given by E. Reissner (Reference 5-1), extended to include the effects of temperature.

The present work is exact within the framework of linear shell theory and removes the following restrictions of the more elementary presentation in Section 7, Volume I (Reference 5-2):

- (1) The material in Volume I was limited to the cases of truncated conical and cylindrical shells.
 - (2) The development in that volume was approximate for the conical shells and,
- (3) The temperature was constant through the thickness, i.e., only meridional variation was permitted.

Since the main objective of this section is to develop the general equations governing the analysis of axisymmetric shells, minimal emphasis is placed upon obtaining specific numerical results. However, problems involving interaction between shells and bulkheads are discussed and, for the purposes of illustration, the general solution for conical and cylindrical shells are derived. Numerical results are given for an unrestrained cylindrical shell subjected to a prescribed temperature variation.

The following symbols are used throughout this section:

h Shell thickness

$$t = \frac{\sqrt{Rh}}{4\sqrt{3(1-\nu)^2}}$$

- r Distance from a general point on the meredian to the axis of revolution
- s Meridional coordinate

5, 2 (Cont[†]d)

Displacement in the r direction u

Axial displacement W Axial coordinate \mathbf{z}

D

Young's modulus

Load - temperature functions defined by Eqs. (1b) of Paragraph 5. 5. 2 and (1b) of G(s), H(s)

Paragraph 5.4.2, respectively

Horizontal force per unit of circumferential length H

K Curvature L Length

M Moment per unit of length Force per unit of length N

 $\int_{h} E\lambda T \zeta d\zeta$, $\int_{h} E\lambda T d\zeta$, respectively Surface traction (force per unit of area)

Herizontal and vertical components of P, respectively

Shear force per unit of circumferential length perpendicular to shell mid-plane Q

 \mathbf{R} Radius of clinder

T Temperature rise above unstressed undeflected datum

V Vertical force per unit of circumferential length

$$\alpha = \sqrt{(r')^2 + (z')^2}$$

β Rotation of meridional element

Strain

3 Thickness coordinate θ Azimuth or hoop angle

λ Coefficient of linear thermal expansion

 ν Poisson's ratio

ξ Meridlonal coordinate

$$\rho = \sqrt[4]{\frac{\text{Ehtan}^2 \mathbf{q}}{D}}$$

Stress σ

 φ Angle between tangent to meridian and the horizontal

SUBSCRIPTS

0, 1 Refer to shell edges

Complementary and particular solution, respectively e, p

ť Initial and final respectively ł, 8 In the meridional direction θ In the azimuth or hoop direction In the meridional direction

5.3 BASIC EQUATIONS

5.3.1 Configuration of a Typical Shell Element

Since this study is concerned with axisymmetric problems for a shell of revolution it is sufficient to consider a typical meridional element before and after deformation due to loads and temperature (Figure 5.3.1-1). Positive directions for loads and displacements are as shown in the figure. The equation of the meridional curve is expressed parametrically by

$$r = r(\xi)$$

$$z = z(\xi)$$
(1a)

where ξ is the independent variable which defines the position of a general point on the meridional mid-surface curve. However, the quantity ξ may not have the units of length (for example, ξ may denote an angle). In order to define length along the curve in a general manner, we introduce the quantity $\alpha = \alpha(\xi)$, such that a differential arc length ds is given by

$$ds = \alpha d\xi. \tag{1b}$$

But,

$$\cos \varphi = \frac{d\mathbf{r}}{d\mathbf{s}} = \frac{d\mathbf{r}}{\alpha d\xi} = \frac{\mathbf{r}^{\dagger}}{\alpha}$$

$$\sin \varphi = \frac{d\mathbf{z}}{d\mathbf{s}} = \frac{d\mathbf{z}}{\alpha d\xi} = \frac{\mathbf{z}^{\dagger}}{\alpha} . \tag{1c}$$

Therefore,

$$(r^{\dagger})^2 + (z^{\dagger})^2 - \alpha^2$$
, (1d)

which defines α .

We consider that the tangent to the meridional curve before deformation makes an angle ϕ with a radial line normal to the axis of revolution; the angle after deformation is designated by $(\phi + \beta)$. Other quantities in the figure are self-explanatory or indicated in the nomenclature, and moment vectors are obtained using the right-hand rule.

5, 3, 2 Equilibrium Equations

In the following derivation the forces per unit of circumferential length are designated by the components (N_{ξ},Q) or the statically equivalent components (V,H). The relationships between these two sets of forces are given by

$$Q = V \cos \varphi - H \sin \varphi$$

$$N_{\xi} = V \sin \varphi + H \cos \varphi$$
(1)

where, in the linear theory, the initial configuration is employed in writing—the force equilibrium equations (terms in β neglected).

A free body diagram showing equilibrium of forces (Figure 5, 3, 2-1) yields the following equations:

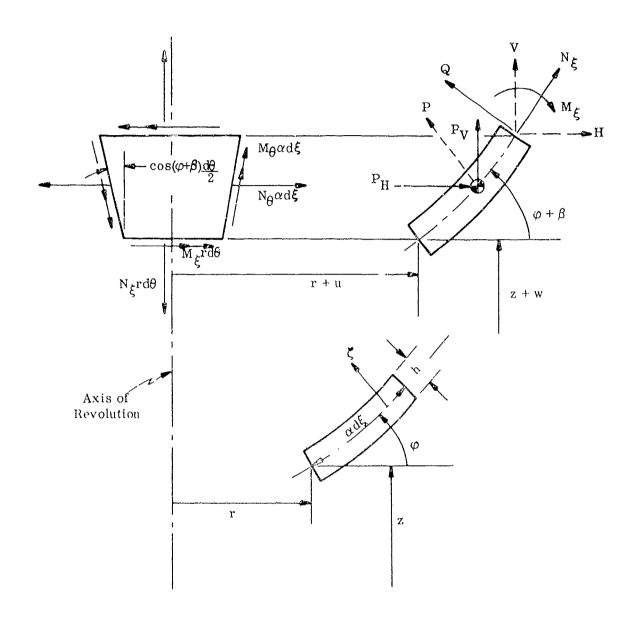


FIGURE 5, 3, 1-1 CONFIGURATION OF A TYPICAL MERIDIONAL ELEMENT BEFORE AND AFTER DEFORMATION

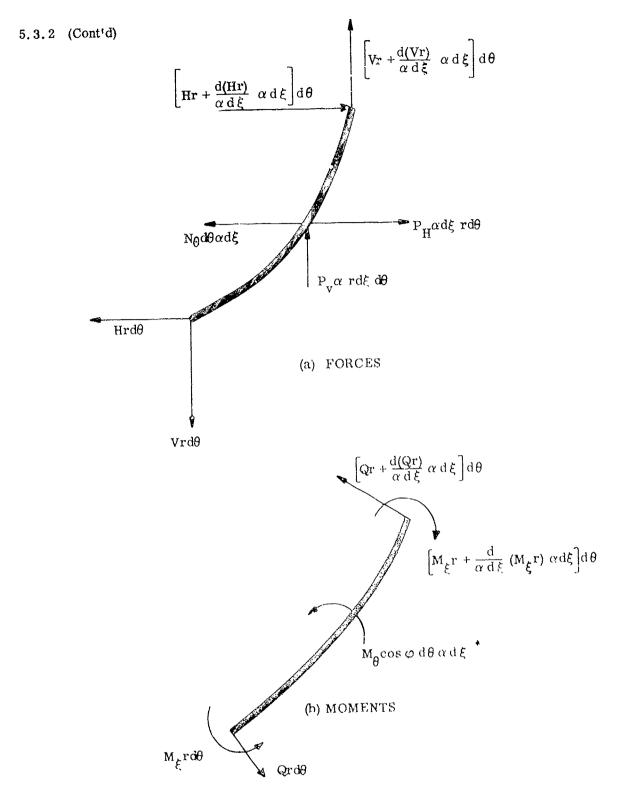


FIGURE 5. 3. 2-1 EQUILIBRIUM OF FORCES AND MOMENTS

^{*} This represents the vector sum of the hoop moments shown in Figure 5.3.1-1. Since the vectors representing the hoop moments form an angle $\cos \varphi d\theta$ (neglecting terms in β), their resultant is of magnitude $\mathbf{M}_{\theta} \cos \varphi d\theta \alpha d\xi$ with the direction as indicated in this figure.

5.3.2 (Cont'd)

$$(\mathbf{r}\mathbf{V})^{\dagger} + \mathbf{r}\alpha\mathbf{P}_{\mathbf{V}} = 0 \tag{2a}$$

$$(rH)' \sim \alpha N_{\theta} + r\alpha P_{H} = 0 . \tag{2b}$$

From the free body diagram of Figure 5. 3. 2-1b, moment equilibrium results in

$$(rM_{\varepsilon})' - \alpha \cos \phi M_{\Theta} - \alpha rQ = 0.$$
 (3)

5, 3, 3 Curvature Changes and Mid-Surface Strains in Terms of Displacements

Consider a meridional element of the shell mid-surface before deformation (Figure 5.3.1-1). The length of this element is given by

$$dL_{i} = \frac{\frac{d\mathbf{r}}{d\xi} d\xi}{\cos \varphi} = \frac{\mathbf{r}^{i} d\xi}{\cos \varphi}.$$

After deformation, the new length is

$$dL_{f} = \frac{(r' + u') d\xi}{\cos (\varphi + \beta)}$$

where β is the rotation of the element. Thus, the meridional strain of the mid-surface is

$$\begin{split} \epsilon_{\xi} &= \frac{\mathrm{d} L_{f} - \mathrm{d} L_{i}}{\mathrm{d} L_{i}} = \frac{\frac{\mathbf{r}^{t} + \mathbf{u}^{t}}{\cos \varphi} - \frac{\mathbf{r}^{t}}{\cos \varphi}}{\frac{\mathbf{r}^{t}}{\cos \varphi}} \\ &= (1 + \frac{\mathbf{u}^{t}}{\mathbf{r}^{t}}) \frac{\cos \varphi}{\cos (\varphi + \beta)} - 1 \\ &= (1 + \frac{\mathbf{u}^{t}}{\mathbf{r}^{t}}) \cos \varphi \left[\frac{1}{\cos \varphi} + \frac{\beta \sin \varphi}{\cos^{2} \varphi} + - - - \right] - 1 \ . \end{split}$$

If we neglect terms higher than order β , the linear meridional midsurface strain becomes*

$$\epsilon_{\xi} = \frac{u^{\dagger}}{r^{\dagger}} + \frac{\sin \varphi}{\cos \varphi} \beta$$

or

$$\epsilon_{\xi} = \frac{\mathbf{u}^{\mathsf{f}}}{\mathbf{r}^{\mathsf{f}}} + \frac{\mathbf{z}^{\mathsf{f}}}{\mathbf{r}^{\mathsf{f}}} \beta . \tag{1}$$

^{*} It should be noted that the derivations in this section involve division by quantities r and r'. Subsequent derivations also involve division by z'. Therefore, at points where these quantities become zero, the resulting derivations and equations become valid only when appropriate limiting processes and regularity conditions are imposed (see Paragraphs 5.4 and 5.5).

5.3.3 (Cont'd)

The hoop strain of the mid-surface ϵ_{θ} is a measure of the increase in circumferential length per unit of length and is therefore given by

$$\epsilon_{\theta} = \frac{\mathbf{u}}{\mathbf{r}}.$$
 (2)

In order to obtain bending moments in terms of deformations, it is necessary to determine the changes in principal curvatures of the shell mid-surface. From the definition of curvature, the change in meridional curvature due to deformation is

$$K_{\xi} = \frac{d(\varphi + \beta)}{\alpha d\xi} - \frac{d\varphi}{\alpha d\xi} = \frac{\beta'}{\alpha}$$
 (3a)

The hoop principal radius of curvature is defined by the length of the normal line to the surface which is bounded by the surface and the axis of revolution. Thus, the change in hoop curvature due to deformation is

$$K_{\theta} = \frac{\sin(\varphi + \beta) - \sin\varphi}{r}$$

Again, neglecting terms of order higher than θ yields

$$K_{\theta} = \frac{\beta \cos \phi}{r} \tag{3b}$$

5, 3, 4 Stress-Strain and Moment-Curvature Relations

The stress-strain relations for the shell mid-surface including temperature are given by

$$\sigma_{\xi} = \frac{E}{1 - \nu^2} \left(\epsilon_{\xi} + \nu \epsilon_{\theta} \right) - \frac{E \lambda T}{1 - \nu}$$

$$\sigma_{\theta} = \frac{E}{1 - \nu^2} \left(\epsilon_{\theta} + \nu \epsilon_{\xi} \right) - \frac{E \lambda T}{1 - \nu} , \qquad (1)$$

where λ is the coefficient of linear thermal expansion. Assuming that the strains are linear through the thickness of the shell, the bending strains are odd functions of ζ and therefore integration of the stress-strain relations through the thickness yields

$$N_{\xi} = \int_{h} \sigma_{\xi} d\zeta = \frac{Eh}{1 - \nu^{2}} \left(\epsilon_{\xi} + \nu \epsilon_{\theta} \right) - \frac{N_{T}}{1 - \nu}$$

$$N_{\theta} = \int_{h} \sigma_{\theta} d\zeta - \frac{Eh}{1 - \nu^{2}} \left(\epsilon_{\theta} + \nu \epsilon_{\xi} \right) - \frac{N_{T}}{1 - \nu}, \tag{2}$$

where

$$N_{\rm T} = \int_{\rm h} E^{\gamma/r} d\zeta$$
.

5.3.4 (Cont'd)

For purposes of obtaining expressions for the moments, it is further assumed that normals to the mid-surface before deformation remain normal after deformation, and that mid-surface stretching due to bending is negligible. Then, employing the stress-strain law, integration of the first moments of the stresses through the thickness results in

$$M_{\xi} = \int_{h} \sigma_{\xi} \zeta d\zeta = -D \left[K_{\xi} + \nu K_{\theta} \right] - \frac{M_{T}}{1 - \nu}$$

$$M_{\theta} = \int_{h} \sigma_{\theta} \zeta d\zeta = -D \left[K_{\theta} + \nu K_{\xi} \right] - \frac{M_{T}}{1 - \nu}$$
(3)

where

$$D = \frac{Eh^3}{12(1 - \nu^2)}$$

$$M_T = \int_h E\lambda T \zeta d\zeta .$$

Substitution of Eqs. (1) - (3) of Paragraph 5, 3, 3 into Eqs. (2) and (3) gives the force resultants and moments in terms of slopes and displacements:

$$N_{\xi} = \frac{Eh}{1 - \nu^{2}} \left[\frac{u^{t} + z^{t} \beta}{r^{t}} + \frac{\nu u}{r} \right] - \frac{N_{T}}{1 - \nu}$$

$$N_{\theta} = \frac{Eh}{1 - \nu^{2}} \left[\frac{u}{r} + \frac{\nu (u^{t} + z^{t} \beta)}{r^{t}} \right] - \frac{N_{T}}{1 - \nu}$$
(4a)

$$M_{\xi} = -D \left[\frac{\beta^{\dagger}}{\alpha} + \frac{\nu \beta r^{\dagger}}{r \alpha} \right] - \frac{M_{T}}{1 - \nu}$$

$$M_{\theta} = -D \left[\frac{\beta r^{\dagger}}{r \alpha} + \frac{\nu \beta^{\dagger}}{\alpha} \right] - \frac{M_{T}}{1 - \nu},$$
(4b)

where $\cos \varphi = \frac{\mathbf{r}^{\mathsf{T}}}{\alpha}$ has been employed.

A compatibility condition in terms of force resultants and slope may now be obtained. Elimination of the displacement u between Eqs. (1) and (2) of Paragraph 3, 3 results in

$$\epsilon_{\xi} = \frac{\left(\frac{\mathbf{r}_{\setminus \theta}}{\mathbf{r}^{\dagger}}\right)^{\dagger}}{\mathbf{r}^{\dagger}} + \frac{\mathbf{z}^{\dagger}}{\mathbf{r}^{\dagger}} \beta . \tag{5a}$$

The above may be combined with Eqs. (2) to yield

5.3.4 (Cont'd)

$$N_{\xi} - \nu N_{\theta} + N_{T} = \frac{\left[r \left(N_{\theta} - \nu N_{\xi} + N_{T} \right) \right]^{\dagger}}{r^{\dagger}} + \frac{Ehz^{\dagger}\beta}{r^{\dagger}}$$
(5b)

5.3.5 Formulation of the Boundary Value Problem

(1) Differential Equation

In the following development, it will be assumed that V is known at one of the shell edges (V = V_0 at $\xi = \xi_0$) so that from static equilibrium Eq. (2a) of Paragraph 5.3.2,

$$rV = r_{o}V_{o} - \int_{\xi_{o}}^{\xi} r \alpha P_{V} d\xi.$$
 (1)

This implies that V is a known quantity throughout the shell.

Then the quantities Q, N_{ξ} , N_{θ} , H, M_{ξ} , M_{θ} may be eliminated from among the seven equations (1), (2a), (2b), (3) of Paragraph 5.3.2, and (4b) and (5b) of Paragraph 5.3.4, to yield a differential equation for the unknown slope β . After detailed calculation and considerable simplification, this equation may be written in the form:

$$\left(\frac{\mathbf{r}}{\mathbf{r}^{\mathsf{T}}}\right)^{2} \left[L(\beta)\right]^{\mathsf{T}} + \frac{1}{\alpha \mathbf{r}} \left[\left(\frac{\alpha \mathbf{r}}{\mathbf{r}^{\mathsf{T}}}\right)^{2} \left(\frac{\mathbf{r}}{\alpha}\right)\right]^{\mathsf{T}} \left[L(\beta)\right]^{\mathsf{T}} + \left[\nu\left(\frac{1}{\alpha}\left(\frac{\alpha \mathbf{r}}{\mathbf{r}^{\mathsf{T}}}\right)\right)^{\mathsf{T}}\right] \\
-i^{\mathsf{T}} - 1 + \frac{1}{r^{\mathsf{T}}}\left(\frac{\mathbf{r}}{\alpha}\left(\frac{\alpha \mathbf{r}}{\mathbf{r}^{\mathsf{T}}}\right)^{\mathsf{T}}\right)^{\mathsf{T}} \left[L(\beta) + \frac{Eh}{D}\frac{z^{\mathsf{T}}}{\mathbf{r}^{\mathsf{T}}}\beta - \frac{\mathbf{r}^{2}}{\alpha z^{\mathsf{T}}}\frac{V^{\mathsf{T}}}{D} + \left[\nu\frac{\mathbf{r}}{\mathbf{r}^{\mathsf{T}}}\left(\frac{z^{\mathsf{T}}}{\alpha}\right)\right] \\
-\frac{z^{\mathsf{T}}}{r}\frac{r^{2}}{\alpha^{\mathsf{T}}}\left(\frac{r^{\mathsf{T}}}{z^{\mathsf{T}}}\right)^{\mathsf{T}}\right] \frac{V^{\mathsf{T}}}{D} + \left[-\left(\frac{\mathbf{r}}{r^{\mathsf{T}}}\right)^{2}\left(\frac{r^{\mathsf{T}^{\mathsf{T}}}}{r^{\mathsf{T}}}\right)^{\mathsf{T}} - \frac{1}{\alpha \mathbf{r}}\left(\frac{\alpha \mathbf{r}}{r^{\mathsf{T}}}\right)^{2}\left(\frac{\mathbf{r}}{\alpha}\right)^{\mathsf{T}}\right] \frac{V}{\alpha z^{\mathsf{T}}}\right] \\
-\left(\left(\frac{\mathbf{r}}{\alpha}\right)\left(\frac{\alpha \mathbf{r}}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\right)^{\mathsf{T}}\left(\frac{r^{\mathsf{T}}}{r^{\mathsf{T}}} + \frac{\alpha}{z^{\mathsf{T}}} + \nu + \frac{1}{r^{\mathsf{T}}}\left(\frac{rz^{\mathsf{T}}}{\alpha}\right)^{\mathsf{T}} - \frac{zr^{\mathsf{T}^{\mathsf{T}}}}{2}\left(\frac{\alpha}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\right] \frac{V}{D} \tag{2}$$

$$+\frac{1}{D}\left[-\nu_{\mathsf{T}}P_{\mathsf{H}} - \frac{1}{r^{\mathsf{T}}}\left(r^{2}P_{\mathsf{H}}\right)^{\mathsf{T}} - \frac{r}{r^{\mathsf{T}}}N_{\mathsf{T}}\right] + \left(\frac{r}{r^{\mathsf{T}}}\right)^{2}\left(\frac{r}{r^{\mathsf{T}}}\right)^{2}\left(\frac{r}{r^{\mathsf{T}}}\right) \frac{M_{\mathsf{T}^{\mathsf{T}}}}{D(1-\nu)}\right] \\
-\frac{1}{\alpha \mathbf{r}}\left(\frac{\alpha \mathbf{r}}{r^{\mathsf{T}}}\right)^{2}\left(\frac{r}{\alpha}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right) \frac{M_{\mathsf{T}^{\mathsf{T}}}}{D(1-r)}\right] - \left[\nu\left(\frac{1}{\alpha}\left(\frac{\alpha \mathbf{r}}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\right)^{\mathsf{T}} - 1\right] - 1$$

$$+\frac{1}{r^{\mathsf{T}}}\left(\frac{r}{\alpha}\left(\frac{\alpha \mathbf{r}}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right) \frac{M_{\mathsf{T}^{\mathsf{T}}}}{D(1-r)}\right) + \frac{1}{r^{\mathsf{T}}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\right] \left(\frac{r^{\mathsf{T}}}{r^{\mathsf{T}}}\right)^{\mathsf{T}} + \frac{1}{r^{\mathsf{T}}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\right] + \frac{1}{r^{\mathsf{T}}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\right] + \frac{1}{r^{\mathsf{T}}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\right] + \frac{1}{r^{\mathsf{T}}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\right] + \frac{1}{r^{\mathsf{T}}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right)^{\mathsf{T}}\left(\frac{r}{r^{\mathsf{T}}}\right$$

5, 3, 5 (Cont'd)

where

$$L(\beta) = \frac{r'}{z'\alpha} 2 \left[\beta^{:} + \frac{\alpha}{r} \left(\frac{r}{\alpha} \right)' \beta' + \left\langle \nu \frac{\alpha}{r} \left(\frac{r'}{\alpha} \right)' - \left(\frac{r'}{r} \right)^2 \right] \beta \right]$$

The Equation (2) is the general differential equation governing the linear analysis for stresses, and deformations in arbitrary shells of revolution due to axisymmetric loads and temperature (see, however, remarks made in footnote, page 5.7).

(2) Force Resultant, Moment, and Displacement Boundary Conditions in Terms of the Slope β .

Since the differential equation (2) is in terms of the dependent variable β , it is advantageous to express the force resultants, moments and displacements, as well as the boundary conditions, as functions of this variable. The derived quantities are given by

$$N_{q} = \frac{D}{\alpha} \left[\frac{\mathbf{r}}{\alpha z^{\dagger}} \beta^{\prime\prime} + \frac{1}{z^{\dagger}} \left(\frac{\mathbf{r}}{\alpha} \right)^{\prime} \beta^{\dagger} + \left\langle \frac{\nu}{z^{\dagger}} \left(\frac{\mathbf{r}^{\dagger}}{\alpha} \right)^{\prime} - \frac{\mathbf{r}^{\dagger 2}}{\alpha \mathbf{r} z^{\dagger}} \right\rangle \beta \right]^{\prime} + \left\langle \frac{\mathbf{r} \mathbf{r}^{\dagger} V}{z^{\dagger}} + \frac{\mathbf{r} M_{T}^{\dagger}}{z^{\dagger} (1 - \nu)} \right\rangle^{\prime} + \mathbf{r} P_{H}$$
(3a)

$$N_{\xi} = D \left[\frac{\mathbf{r}'}{\alpha^{2} \mathbf{z}'} \beta^{\prime\prime} + \frac{\mathbf{r}'}{\mathbf{r} \alpha \mathbf{z}'} \left(\frac{\mathbf{r}}{\alpha} \right)' \beta^{\prime\prime} + \frac{\mathbf{r}'}{\mathbf{r} \alpha} \left\langle \frac{\mathbf{r}'}{\mathbf{z}'} \left(\frac{\mathbf{r}'}{\alpha} \right)' - \frac{\mathbf{r}'^{2}}{\alpha \mathbf{r} \mathbf{z}'} \right\rangle \beta \right]$$

$$+ \frac{\alpha}{\mathbf{z}'} V + \frac{\mathbf{r}'}{\alpha \mathbf{z}'} \frac{M_{\mathbf{T}'}}{1 - \nu}$$
(3b)

$$Q = -D\left[-\frac{1}{\alpha^{2}}\beta^{*} + \frac{1}{r\alpha}\left(\frac{r}{\alpha}\right)^{t}\beta^{t} + \left\langle\frac{\nu}{r\alpha}\left(\frac{r^{t}}{\alpha}\right)^{t} - \left(\frac{r^{t}}{\alpha r}\right)^{2}\right\rangle\beta\right] - \frac{M_{T}^{t}}{\alpha(1-\nu)}$$
(3c)

$$H = \frac{D}{\alpha z^{\mathsf{f}}} \left[\beta^{\mathsf{i}^{\mathsf{f}}} + \frac{\alpha}{r} \left(\frac{r}{\alpha} \right)^{\mathsf{f}} \beta^{\mathsf{f}} + \left\langle \frac{\nu \alpha}{r} \left(\frac{r^{\mathsf{f}}}{\alpha} \right)^{\mathsf{f}} - \left(\frac{r^{\mathsf{f}}}{r} \right)^{2} \right\rangle \beta + \frac{\alpha r^{\mathsf{f}} V}{D} + \frac{\alpha M_{\mathsf{T}}^{\mathsf{f}}}{D(1 - \nu)} \right]$$
(3d)

$$\mathbf{M}_{\xi} = -\mathbf{D} \left[\frac{\beta^{\dagger}}{\alpha} + \frac{\nu \mathbf{r}^{\dagger}}{\mathbf{r}\alpha} \beta \right] - \frac{\mathbf{M}_{\Upsilon}}{1 + \nu}$$
 (3e)

5.3.5 (Cont'd)

$$M_{\theta} = -D \left[\frac{r^{\dagger}}{r\alpha} \beta + \frac{\nu \beta^{\dagger}}{\alpha} \right] - \frac{M_{T}}{1 - \nu}$$
 (3f)

$$u = \frac{\mathbf{r}}{Eh} \left(N_{\theta} - \nu N_{\xi} \right) + \frac{N_{T}^{\mathbf{r}}}{Eh}$$
 (3g)

$$\mathbf{w} = \int_{\xi_{\mathbf{o}}}^{\xi} \left[\frac{\mathbf{z}^{\dagger}}{\mathbf{E} \mathbf{h}} \left(\mathbf{N}_{\xi} - \nu \mathbf{N}_{\theta} + \mathbf{N}_{T} \right) + \mathbf{r}^{\dagger} \boldsymbol{\beta} \right] d\xi.$$
 (3h)

The stresses may then be obtained from

$$\sigma_{\xi} = \frac{1}{h} \left(N_{\xi} + \frac{N_{T}}{1 - \nu} \right) - \frac{E\lambda T}{1 - \nu} + \frac{12\zeta}{h^3} \left(M_{\xi} + \frac{M_{T}}{1 - \nu} \right)$$
 (4a)

$$\sigma_{\theta} = \frac{1}{h} \left(N_{\theta} + \frac{N_{T}}{1 - \nu} \right) - \frac{E\lambda T}{1 - \nu} + \frac{12\zeta}{h^3} \left(M_{\theta} + \frac{M_{T}}{1 - \nu} \right) . \tag{4b}$$

Typical boundary conditions can be expressed in terms of the above derived quantities as shown by the following examples:

(a) Clamped Edge at
$$\xi = \xi_0$$

$$\begin{bmatrix} \beta \end{bmatrix} \xi = \xi_0 = \begin{bmatrix} u \end{bmatrix} \xi = \xi_0$$
(5a)

(b) Pinned Edge at $\xi = \xi_0$ $\left[u \right]_{\xi = \xi} = \left[M_{\xi} \right]_{\xi = \xi} = 0$ (5b)

(d) Specified Radial Load "H $_{o}$ " and Meridianal Moment "M $_{o}$ " at edge ξ = ξ

$$\left[\frac{D}{\alpha \mathbf{z}^{\dagger}} + \mathbf{s}^{\prime\prime} + \frac{\alpha}{\mathbf{r}} \left(\frac{\mathbf{r}}{\alpha}\right)^{\dagger} \beta^{\prime\prime} + \left\{\frac{v\alpha}{\mathbf{r}} \left(\frac{\mathbf{r}^{\prime}}{\alpha}\right)^{\prime\prime} - \left(\frac{\mathbf{r}^{\prime\prime}}{\mathbf{r}}\right)^{2}\right\} \beta$$

$$+ \frac{\alpha \mathbf{r}^{\prime\prime} \mathbf{V}}{D} + \frac{\alpha \mathbf{M}_{\mathbf{T}^{\prime}}}{D(1-v)} + \int_{\xi = \xi_{0}} H_{0}$$

$$\left[-D + \frac{\beta^{\prime\prime}}{\alpha} + \frac{\nu \mathbf{r}^{\prime\prime}}{\mathbf{r}\alpha} \beta - \frac{\mathbf{M}_{\mathbf{T}^{\prime\prime}}}{1-v}\right]_{\xi = \xi_{0}} M_{0}$$
(5d)

5.3.5 (Cont'd)

In order to apply the differential equation (2) and boundary conditions (5) to a specific shell geometry, the quantities r, α , z must be determined from the equation of the shell meridian. Specific examples for the cases of conical and cylindrical shells are presented in the following paragraphs. In general, the majet problem encountered in solving Eq. (2) for a given shell of revolution involves obtaining the complementary solution for a fourth order differential equation with variable coefficients. Usually crosed form solutions are not possible. However, as may be observed from an inspection of the right hand side of Eq. (2), the introduction of temperature does not complicate matters, since it enters in a form analogous to terms resulting from mechanical loads.

The boundary conditions (Eq. (5d)) are of importance in considering the interaction of the shell with both transverse circular bulkheads and other shells of revolution. Once the linear response to edge load, and moments is determined from a solution of Eq. (2) subject to boundary conditions of the type (5d), influence coefficients are known and internal loads at the junctions of shells and bulkheads may be evaluated by imposing compatibility and equilibrium conditions on slopes and deflections. A description of the procedure to be followed is given in detail in Volume I. Section 8.

5.4 CONICAL SHELLS

5, 4, 1 Basic Equations

A meridional section of the cone and the coordinate system is shown in Figure 5.4.14.

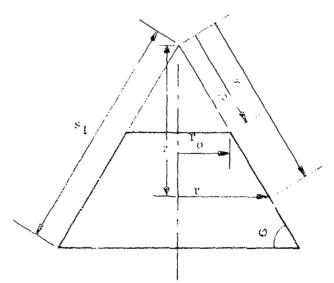


FIGURE 5, 4, 1-1 CONE GEOMETRY

Referring to Eqs. (1) of Paragraph 5.3.1, we choose the independent variable as

$$\xi = \mathbf{s}$$

5.4.1 (Cont'd)

from which it follows that

$$r = s \cos \phi$$

$$z = s \sin \phi$$

$$r' = \cos \phi$$

$$z' = \sin \phi$$

$$\alpha = 1$$
, (1b)

where company has been employed and primed quantities indicate differentiation with respect to s.

The above quantities may be substituted into the general differential equation (2) of Para graph 5, 3, 5. After simplification, the vesulting differential equation for the linear analysis o' confeat shells becomes

$$88^{W} + 48^{m} + 6^{\frac{1}{8}} \frac{3}{8} = \frac{1}{6} \left[\frac{1}{8} \left(P_{V} s^{2} \right)^{4} + \nu P_{W} \cos \phi \right]$$

$$+ \frac{1}{\cos \phi} \frac{1}{8} \left[\frac{8}{8} + \frac{1}{6} \left(P_{V} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{\cos \phi} \frac{1}{8} \frac{1}{8} \left[\frac{8}{8} + \frac{1}{4} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{\cos \phi} \frac{1}{6} \left[\frac{8}{8} + \frac{1}{4} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{\cos \phi} \frac{1}{6} \left[\frac{8}{8} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{\cos \phi} \frac{1}{6} \left[\frac{8}{8} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{\cos \phi} \frac{1}{6} \left[\frac{8}{8} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{\cos \phi} \frac{1}{6} \left[\frac{8}{8} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{\cos \phi} \frac{1}{6} \left[\frac{8}{8} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{\cos \phi} \frac{1}{6} \left[\frac{8}{8} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{\cos \phi} \frac{1}{6} \left[\frac{1}{6} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{\cos \phi} \frac{1}{6} \left[\frac{1}{6} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{6} \frac{1}{6} \frac{1}{6} \left[\frac{1}{6} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{6} \frac{1}{6} \frac{1}{6} \left[\frac{1}{6} + \frac{1}{6} \left(P_{W} s^{2} \right)^{4} + \nu P_{W} \sin \phi \right]$$

$$+ \frac{1}{8} \frac{1}{6} \frac{1}{6}$$

where

We will consider the special case of tormal partage pressure. Then, from Figure 5, 3, 1-1:

$$P_{V} = P \cos \phi$$

 $P_{H} = -P \sin \phi$,

and Eq. (2) reduces to

$$ss^{IV} + 4s^{2m+1} o^{A} \frac{\beta}{s} = \frac{1}{p} \frac{1}{2s} (Fs^{2})' + \frac{1}{2} \frac{s}{s^{2}} \frac{s}{s^{2}} Psds + \frac{r_{0}V_{0}}{s^{2} \cos^{2} o} tance N_{1}'$$

$$+ \frac{1}{s^{2}} \left(s^{3} \frac{M_{1}r_{0}'}{V_{0}r_{0}} \right) = .$$
(2)

5.4.1 (Cont'd)

Correspondingly, the expressions for the force resultants and mome its are obtained from Eqs. (3) of Paragraph 5.3.5. Tare given by

$$N_{\theta} = \frac{1}{\tan \varphi} \left[D \left(s \beta'' + \beta' - \frac{\beta}{s} \right)' - Ps + \frac{\left(s M_{T}' \right)'}{1 - \iota'} \right]$$
(4a)

$$N_{s} = \frac{1}{\tan \varphi} \left[D\left(3^{\prime\prime} + \frac{3^{\prime}}{s} - \frac{8}{s^{2}}\right) + \frac{r_{o}V_{o}}{s^{cos}^{2}} - \frac{1}{s} \left[\frac{s}{s} p_{sds} + \frac{M^{\prime}T}{1 - \nu} \right] \right]$$
(4b)

$$Q = -D \left[-3^{n} + \frac{8^{n}}{s} - \frac{3}{s^{2}} \right] - \frac{M^{n}}{1 - U}$$
 (4c)

$$H \approx \frac{P}{\sin \phi} \cdot \left[\beta^{n} + \frac{\beta^{n}}{s} + \frac{\beta}{s} \right] \cdot \frac{V_{0} \varepsilon_{0}}{s \sin \phi} + \frac{\cos^{2} \phi}{s \sin \phi} \cdot \frac{s}{s} \frac{P_{\text{sids}} + \frac{1}{\sin \phi} \frac{M^{n}}{1 + 1}}{s \sin \phi} \right]$$
(4.5)

$$\mathcal{M}_{S} = -D \left[\beta' + \frac{\nu \beta}{\beta} \right] = \frac{M_{T}}{1 - \nu} \tag{4e}$$

$$\mathbf{M}_{G} = -D\left[\frac{3}{8} + \nu B^{\dagger}\right] = \frac{M_{\odot}}{6.3} \tag{40}$$

5, 4.2 Solution of the D'Aerential Equation?

When Eq. (3) of Paragraph 5, 4, 1 is multiplied by s, we have

$$s^{2} S^{1V} + 4s \beta^{m} + o^{2} S + H(s) + \frac{r V}{\sigma^{2} \sigma^{2}} , \qquad (10)$$

where

$$H(s) = \frac{1}{B} \left[\left(Ps^2 \right)^2 + \frac{1}{8} \left(\frac{s}{8} \right)^8 + \frac{1}{8} \left(\frac{3}{8} \frac{M^2 \gamma}{Y + \gamma} \right) + 16B \varphi + N^2 \gamma \right]. \tag{1b}$$

Introduce a new independent variable; defined by

$$z = 2c_0$$

^{*} This differential equation? It comes of revolution subjected to axisyometria mechanical loads and temperature was solved by Eut? (Reference 5-5) where reference is ziven to prove tous work on the subject. However, the equations did not proceed from any general development on shells of revenition. For Isomote, only linear temperature radiations through the thickness are accommodated in this reference.

5.4.2 (Cont'd)

Then the homogeneous form of Eq. (1) becomes

$$\frac{d^{4}\beta_{c}}{d\psi^{4}} + \frac{2}{\psi} \frac{d^{3}\beta_{c}}{d\psi^{3}} - \frac{9}{\psi^{2}} \frac{d^{2}\beta_{c}}{d\psi^{2}} + \frac{9}{\psi^{3}} \frac{d\beta_{c}}{d\psi} + \beta_{c} = 0.$$
 (3a)

This equation is now factorable as

$$\left(\frac{\mathrm{d}^2}{\mathrm{d}\psi^2} + \frac{1}{\psi} \frac{\mathrm{d}}{\mathrm{d}\psi} - i - \frac{4}{\psi^2}\right) \left(\frac{\mathrm{d}^2}{\mathrm{d}\psi^2} + \frac{1}{\psi} \frac{\mathrm{d}}{\mathrm{d}\psi} - i - \frac{4}{\psi^2}\right) \beta_{\mathbf{c}} = 0, \tag{3b}$$

for which the solution is

$$\beta_{c} = c_{1} J_{2} \left(i^{\frac{1}{2}} \psi \right) + c_{2} Y_{2} \left(i^{\frac{1}{2}} \psi \right) + c_{3} J_{2} \left(i^{\frac{3}{2}} \psi \right) + c_{4} Y_{2} \left(i^{\frac{3}{2}} \psi \right), \tag{4}$$

where J_2 and Y_2 are Bessel functions of the first and second kind of order 2. After much manipulation Eq. (4) may be written in terms of Kelvin functions as

$$\beta_{c} = A \left(ber \psi - \frac{2}{\psi} \dot{b}ei \psi \right) + B \left(bei \psi + \frac{2}{\psi} \dot{b}er \psi \right)$$

$$+ C \left(ker \psi - \frac{2}{\psi} \dot{k}ei \psi \right) + F \left(kei \psi + \frac{2}{\psi} \dot{k}er \psi \right)$$
(5)

where $\cdot = \frac{d}{d\psi}$.

The particular solution of Eq. (1) can be easily obtained if the function H(s) is expressed as a polynomial of the form

$$H(s) = \sum_{p=0}^{N} A_p s^p \tag{6}$$

Then, selecting the solution on the form of series involving integral powers of s, there results

$$\beta_{p} = \frac{r_{o}V_{o}}{Ehs \sin^{2}\varphi} + \frac{A_{o}}{\rho^{4}}$$

$$+ \sum_{p=1}^{1} \sum_{m=1}^{p} \left[\frac{\langle -1 \rangle^{p-m} + 1}{2} \right] a_{m}s^{m}, \qquad (7)$$

5.4.2 (Contid)

where the am are given by

$$a_{m} = (-1)^{\frac{p-m}{2}} \frac{A_{p}(p+1) (p) (m+1) (m)}{\rho^{2} (p-m+2)} \left[\frac{(p-1)!}{(m+1)!} \right]^{2}$$
(8)

The total solution to the cone problem is $\beta = \beta_c + \beta_p$ and the constants A, B, C, F must be determined from the four boundary conditions (two at each edge). For the special case of a full cone (s₀ = 0), finiteness of stresses at the apex requires that $V_0 = 0$. Further, regularity conditions on stress resultants and shears, require that "C" = "F" = 0. Specific solutions for the full cone then resolve themselves into the determination of the two constants "A" and "B" from the boundary conditions at the base.

The Kelvin functions and their first derivatives appearing in Eq. (5) are extensively and accurately tabulated in Reference 5-4 for a wide range of the argument. The form of solution developed above is readily adaptable to numerical computation using digital computers. Parametric studies to determine stresses and deflections can be made.

5.5 CYLINDRICAL SHELLS

5.5.1 Basic Equations

The cylinder and the coordinate system is shown in Figure 5. 5. 1-1.

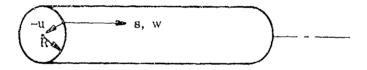


FIGURE 5.5.1-1 CYLINDRICAL GEOMETRY

We choose s again as the independent variable, however, since r' = 0 for the cylinder (see Figure 5.3.1-1), then as indicated previously Eq. (2) of Paragraph 5.3.5 is not applicable in the form given. The equations for the cylindrical shell are more readily derivable from the conical shell equations (which were developed from Eq. (2) of Paragraph 5.3.5). Proceeding in this manner, if the equation (3) of Paragraph 5.4.1 is multiplied by $\frac{D}{s}$, the quantity $\frac{1}{s}$ replaced by $\frac{\cos \phi}{r}$ and the appropriate limit as $\phi \rightarrow \frac{\pi}{2}$, $r \rightarrow R$, is taken, there results, for the case of normal pressures,

$$D\beta^{IV} + \frac{Eh\beta}{R^2} = \left[P - \frac{M_T''}{1-\nu} - \frac{N_T}{R} \right]' . \tag{1}$$

5, 5, 1 (Cont'd)

Similarly, the limiting process applied to Eqs. (4) of Paragraph 5.4.1 for the force and moment resultants yields

$$N_{\theta} = R \left[D\beta^{\dagger \dagger \dagger} + \frac{M_{T}^{\dagger \dagger}}{1 - \nu} - P \right]$$

$$N_{g} = V_{o}$$

$$Q = -\left[D\beta^{\dagger \dagger} + \frac{M_{T}^{\dagger}}{1 - \nu} \right] = -H$$

$$M_{g} = -D\beta^{\dagger} - \frac{M_{T}^{\dagger}}{1 - \nu}$$

$$M_{\theta} = -\nu D\beta^{\dagger} - \frac{M_{T}^{\dagger}}{1 - \nu} .$$
(2)

An alternate form of the equilibrium equation in terms of the radial displacement component "u" can be obtained by integrating Eq. (1) once, noting, that $\beta = -u$. This results in

$$Du^{TV} + \frac{Eh}{R^2}u = -P + \frac{M_T''}{1-\nu} + \frac{N_T}{R} - \frac{\nu V_0}{R}, \qquad (3)$$

where the constant of integration is given by the last term on the right. Substituting $\beta = -u^t$ into Eqs. (2) the force and moment resultants are

$$N_{\theta} = R \left[-Du^{IV} + \frac{M_{T}^{"}}{1 - \nu} - P \right]$$

$$N_{s} = V_{o}$$

$$Q = Du^{"} - \frac{M_{T}^{"}}{1 - \nu} = -H$$

$$M_{s} = Du^{"} - \frac{M_{T}}{1 - \nu}$$

$$M_{\theta} = \nu Du^{"} - \frac{M_{T}}{1 - \nu}.$$

$$(4)$$

Using the formulation of Eq. (1), for all sets of boundary conditions involving β , the solution may be integrated once to obtain u. The constant of integration is determined from

5.5.1 (Cont'd)

the prescribed value of V_0 . When using the formulation of Eq. (3), however, V_0 appears explicitly in the differential equation.

5.5.2 Solution of the Differential Equation

Equation (3) may be written as

$$\frac{\ell^4}{4} u^{IV} + u = G(s) \tag{1a}$$

where

$$\ell = \frac{\sqrt{Rh}}{4\sqrt{3(1-\nu)^2}}$$
 (= .778 \sqrt{Rh} for ν = .30).

and

G(s) =
$$\frac{R^2}{Eh} \left[-P + \frac{M_T''}{1-\nu} + \frac{N_T}{R} - \frac{\nu V_o}{R} \right]$$
. (1b)

The complementary solution of (1a) is given by

$$u_{c} = \sinh \frac{8}{\ell} \left(A_{1} \sin \frac{8}{\ell} + A_{2} \cos \frac{8}{\ell} \right) + \cosh \frac{8}{\ell} \left(A_{3} \sin \frac{8}{\ell} + A_{4} \cos \frac{8}{\ell} \right). \tag{2}$$

A particular solution can easily be obtained if G(s) is expressed as a polynomial with terms of the form

$$G_k(s) = C_k \left(\frac{s}{R}\right)^k$$
; $k = 0, 1, 2 \dots M.$ (3)

Then a particular solution corresponding to $G(s) = C_k \left(\frac{s}{R}\right)^k$ can be determined by assuming this solution in the form of a polynomial in (s/R) of degree K and substituting in Eq. (1). There results

5.5.2 (Cont'd)

where

$$A_{(k-4j)} = (-1)^{j} C_{k} \frac{(k)!}{(k-4j)!} \left[\frac{1}{4} \left(\frac{\ell}{R} \right)^{4} \right]^{j},$$

and

$$[N] = Greatest integer \le \frac{k}{4}$$
.

The complete solution corresponding to G(s) = $\sum_{k=0}^{M} C_k \left(\frac{s}{R}\right)^k$ is then given by

$$u = u_c + \sum_{k=0}^{M} (u_p)_k . \tag{5}$$

The constants A_1 , A_2 , A_3 , A_4 must be determined from the specified boundary conditions at the ends of the cylinder. This requires the solution of four simultaneous, linear algebraic equations. In order to eliminate the necessity of solving four equations an approximate method of solution in which the less tedious procedure of solving two pairs of simultaneous equations, each for two unknown constants, is used extensively. The basis of this method is the assumption that edge shears and moments (which are self equilibrating applied at one end of the shell negligibly affect the stresses and deflections at the opposite end. This assumption is valid if the length of the shell "L" satisfies the condition $L \ge 3\ell$, or equivalently, for $\nu = 0.30$, when

 $\frac{L}{R} \ge \frac{7}{3} \left(\frac{h}{R}\right)^{1/2}$. The inequality is satisfied in all but very short/thick cylindrical shells. The method, which appears in Reference 5-2, may be described as follows:

Determine the deflections and stresses corresponding to the particular solution $u_p = \sum_{k=0}^{M} (u_p)_k$ where the $(u_p)_k$ are given by Eq. (4) In general this solution will not satisfy the boundary conditions.

(2) Edge moments and loads are then applied to the semi-infinite cylinder such that when their effects are superposed on the solutions of (1) the boundary conditions at each edge are satisfied. Since there are only two conditions to be met at each edge and there is no assumed interaction of edge effects then, for each edge, the solution of two simultaneous equations in two unknowns (one shear and one bending moment) is required.

5.5.3 NUMERICAL EXAMPLE

A free cylindrical shell is subjected to an elevated temperature as shown in Figure 5.5.3-1, i.e., the inside surface of the cylinder is at a uniform elevated temperature T_i and the outside temperature varies linearly from T_i at set to a temperature T_i (I+A) at set. The

5.5.3 (Cont[†]d)

variation through the thickness of the shell is assumed to be linear. It is required to determine the stresses and deflections.

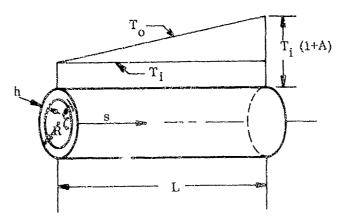


FIGURE 5.5.3-1 ILLUSTRATIVE EXAMPLE

Procedure:

The temperature distribution is given by

$$T(s, \zeta) = T_i \left[1 + \frac{As}{2L} \left(1 - \frac{2\zeta}{h} \right) \right]$$

and therefore

$$N_{T} = E\lambda \int_{-\frac{h}{2}}^{\frac{h}{2}} Td\zeta = E\lambda T_{i} h \left(1 + \frac{As}{2L}\right)$$

$$M_{T} = E\lambda \int_{-\frac{h}{2}}^{\frac{h}{2}} T\zeta dz = -E\lambda T_{i} \frac{As}{L} \frac{h^{2}}{12}$$

Referring to Eq. (1b) of Paragraph 5. 5. 2, since no mechanical loads are applied, $P = V_0 = 0, \text{ and Eq.(1a) of Paragraph 5. 5. 2 becomes } \frac{\frac{1}{4}}{4} u^{TV} + u = \frac{R^2}{Eh} \left[\frac{M_T^{(i)}}{1 - \nu} + \frac{N_T}{R} \right]$ $= R\lambda T_i \left(1 + \frac{\Lambda s}{2L} \right).$

The right hand side can be written in the form $C_o + C_1 = \frac{8}{R}$,

where

$$C_0 = R\lambda T_1$$

$$C_1 = \frac{R^2 \lambda T_1 \Lambda}{2L} .$$

5.5.3 (Cont'd)

Then from Eqs. (2), (4), and (5) of Paragraph 5.5.2 the solution for the displacement is given by

$$u = \sinh \frac{s}{t} \left(A_1 \sin \frac{s}{t} + A_2 \cos \frac{s}{t} \right) + \cosh \frac{s}{t} \left(A_3 \sin \frac{s}{t} \right)$$

$$+ A_4 \cos \frac{s}{t} + R\lambda T_i \left(1 + \frac{As}{2L} \right) .$$
(1)

The boundary conditions for free edges are

$$M_s = Q = 0$$
 at $s = 0$, L,

which from Eqs. 4 of Paragraph 5.5.1 can be written as

$$Du'' \sim \frac{M_T}{1-\nu} := Du''' - \frac{M_T'}{1-\nu} := 0 \text{ at } s = 0, L.$$
 (2)

The constants $A_1 = A_4$ in Eq. (1) are determined from the boundary conditions.

Force resultants and moments are then found by substituting into Eqs. (2) of Paragraph 5.5.1. Since the calculation details are straightforward, only the results are shown. Figure 5.5.3-2 gives nondimensional deflections, force and moment resultants for both a cylinder with $L/\ell \approx 10$ and a longer cylinder corresponding to $L/\ell \approx 50$. The graphs show that as the cylinder becomes longer, the peak deflections and stresses approach the end of the surface subjected to the higher thermal gradient. In general, these peak values tend to increase in magnitude for the longer cylinders, resulting in sharp gradients in the vicinity of the edge.

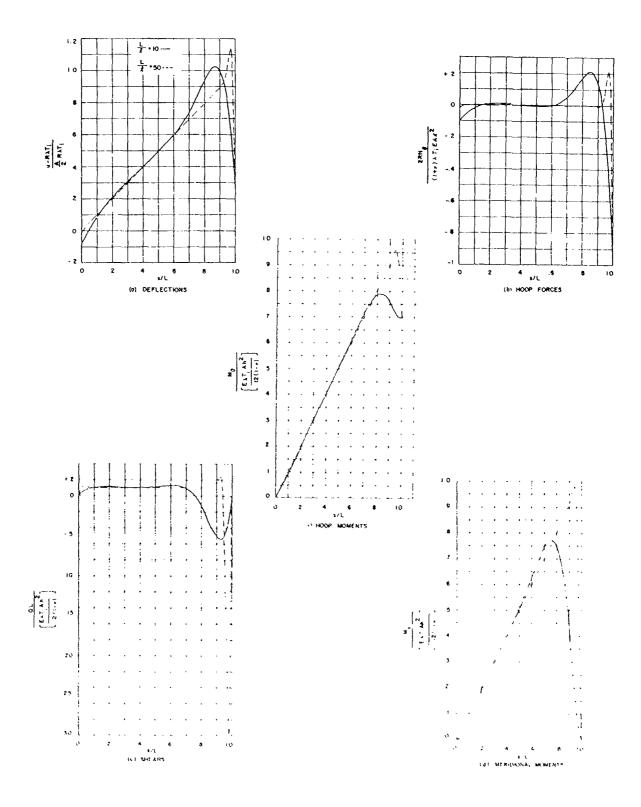


FIGURE 5,5,3-2 NONDIMENSIONAL DEFLECTIONS, FORCE AND MOMENT RESULTANTS FOR THE FREE CYLINDER HEATED AS SHOWN IN FIGURE 5.5.3-1: 1.7t: 10.50.

5.6 REFERENCES

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5. Mochanical stresses
I. AFSC Project 1367,
Task 136710

III. Republic Aviation Corp., R & D Div., Farmingdale, N.Y. AP33(616)~6066 II. Contract

IV. M.J. Forray,

In ASTIA collection V. RAC 128-1 VI. Avel fr OTS VII. In ASTIA coll

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